STRUCTURAL NOTES

ELECTRONIC VERSIONS OF STRUCTURAL DRAWINGS ARE THE SOLE, COPYRIGHTED PROPERTY OF KARINS ENGINEERING GROUP INC. ELECTRONIC VERSIONS SHALL NOT BE TRANSFERRED OR SHARED WITHOUT THE EXPRESS, WRITTEN PERMISSION OF KARINS ENGINEERING GROUP INC.

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES. DEPRESSIONS. AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

DIMENSIONS AND EXISTING CONDITIONS THROUGHOUT THE PROJECT SHALL BE FIELD VERIFIED. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER—OF—RECORD BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.

MODIFIED STRUCTURAL MEMBERS ARE IN COMPLIANCE WITH FLORIDA BUILDING CODE 2007 EDITION OF THE FLORIDA BUILDING CODE W/ REVISION 2009. MEMBERS NOT DESIGNATED FOR ALTERATIONS WERE ASSUMED ADEQUATE. NO EXHAUSTIVE STUDIES WERE PERFORMED BY KARINS ENGINEERING GROUP, INC. (KEG) PRIOR TO THIS DESIGN TO UNCOVER ALL CONSTRUCTION DEFECTS OR CODE VIOLATIONS.

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO INSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS.

NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL OR FOUNDATION MEMBERS OTHER THAN THOSE LOCATED ON THESE DRAWINGS WITHOUT PREVIOUS APPROVAL OF THE ENGINEER.

PROPOSED CHANGES AND REQUEST FOR INFORMATION OR SUBSTITUTION SHALL BE SUBMITTED TO THE ARCHITECT OF RECORD. ARCHITECT OF RECORD SHALL REVIEW AND APPROVE PROPOSAL OR REQUEST PRIOR TO SUBMITTING TO ENGINEERING OF RECORD. ENGINEER OF RECORD SHALL REVIEW AND RETURN TO ARCHITECT BEFORE CHANGES IN-FIELD PROCEED.

DESIGN LOADS:

THE STRUCTURAL SYSTEM FOR BUILDING DEPICTED HEREON HAS BEEN DESIGNED ACCORDING TO THE 2010 EDITION OF THE FLORIDA BUILDING CODE.

THE STRUCTURE HAS BEEN DESIGNED TO RESIST THE FOLLOWING SUPERIMPOSED LOADS.

FLOOR:
LIVE LOAD: 40 psf
ROOF:
LIVE LOAD: 20 psf
DEAD LOAD: 25 psf

WIND:

ASCE 7-10: DESIGN WIND SPEED Vult=150 mph, Vasd=116 mph 3-SEC. GUST, RISK CATEGORY II, EXPOSURE CATEGORY D, OPEN BUILDING (CANOPY), PARTIALLY ENCLOSED BUILDING (RESTROOM), MAIN WIND FORCE RESISTING SYSTEM ROOF DESIGN PRESSURES: +34.3 psf, -72.9 psf (ULTIMATE). GCpi = ±0.55.

CONCRETE REINFORCING:

SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, U.N.O, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SHOP DRAWINGS DEPICTING QUANTITY AND INTENDED PLACEMENT LOCATION OF REINFORCING STEEL SHALL BE SUBMITTED PRIOR TO COMMENCEMENT OF FABRICATION.

CONCRETE:

CONCRETE SHALL BE AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX: 3000 psi FOR SLAB ON GRADE, STEPS AND ALL NON-ELEVATED FLATWORK 5000 psi FOR BEAMS; WATER TO CEMENTITIOUS RATIO NOT TO EXCEED 0.4.

SOIL TO BE COMPACTED TO 95% MODIFIED PROCTOR DENSITY DETERMINED IN ACCORDANCE WITH ASTM D-1557.

CONCRETE SHALL COMPLY WITH THE REQUIREMENTS OF ASTM C-94 FOR MEASURING, MIXING, TRANSPORTING, ETC. MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. WATER/ CEMENT RATIO FOR CONCRETE AT EXPOSED TO CHLORIDES & DECKS SHALL NOT EXCEED 0.40 BY WEIGHT AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 5000psi.

CONCRETE SHALL COMPLY WITH ALL THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED. CONCRETE SHALL BE PLACED IN ITS FINAL POSITION WITHIN 90 MINUTES AFTER THE ADDITION OF BATCH WATER. CONCRETE SHALL BE DISCARDED IF THE FOREGOING ELAPSED TIME IS EXCEEDED. THE ON—SITE REPRESENTATIVE OF THE TESTING LAB SHALL NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NOTED NONCOMPLIANCE WITH THE ABOVE.

SLABS SHALL BE MOIST CURED OR CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1 AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. ALL SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY. CALCIUM CHLORIDES SHALL NOT BE UTILIZED; OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER. PREPARATION SHALL BE IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS.

PENETRATIONS:

UNLESS CLEARLY SHOWN ON THE STRUCTURAL DRAWINGS, NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL OR FOUNDATION MEMBER WITHOUT A PRIOR WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER. IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO IDENTIFY SUCH PENETRATIONS BASED ON INFORMATION PROVIDED BY ALL SUBCONTRACTORS AND TO SUBMIT DETAILED AND DIMENSIONED FLOOR PLANS TO THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL PRIOR TO IMPLEMENTATION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ANY LABOR AND MATERIALS ASSOCIATED WITH ADDITIONAL REINFORCEMENT OF STRUCTURAL MEMBERS RESULTING FROM INTRODUCTION OF SUCH PENETRATIONS.

CHEMICAL ANCHORS:

ALL CHEMICAL ANCHORS SHALL BE SUBMITTED FOR ENGINEER APPROVAL PRIOR TO INSTALLATION. CHEMICAL ANCHORS SHALL BE SUBMITTED FOR EACH SPECIFIC USE AND CONDITION.

MASONRY WALLS:

MASONRY UNITS SHALL MEET ASTM C-90 FOR HOLLOW LOAD BEARING TYPE MASONRY WITH UNIT STRENGTH OF 1900 psi ON THE NET AREA (f'm = 1500 psi). MORTAR SHALL BE TYPE "M" OR "S" AND MEET ASTM C-270 WITH MINIMUM COMPRESSION STRENGTH OF 1900 psi OR TYPE "S" OR 2500 psi FOR TYPE "M". GROUT SHALL BE 2500 psi MINIMUM COMPRESSIVE STRENGTH AND MEET ASTM C-1019.

REINFORCED VERTICAL CELLS AS SHOWN ON PLANS SHALL BE GROUT FILLED WITH 1#5 REINFORCING BAR VERTICAL, UNLESS OTHERWISE NOTED. EXCEPT AS NOTED VERTICAL REINFORCEMENT SHALL BE PROVIDED AT CORNERS, INTERSECTIONS, WITHIN 16 IN. OF EACH SIDE OF OPENINGS, WITHIN 8 IN. OF EACH SIDE OF MOVEMENT JOINTS, WITHIN 8 IN. OF THE ENDS OF THE WALLS, AND AT A MAXIMUM SPACING OF 4 FEET ON CENTER. REINFORCEMENT ADJACENT TO OPENINGS NEED NOT BE PROVIDED FOR OPENINGS SMALLER THAN 16 IN. IN EITHER THE HORIZONTAL OR VERTICAL DIRECTION, UNLESS THE SPACING OF DISTRIBUTED REINFORCEMENT IS INTERRUPTED BY SUCH OPENINGS. HORIZONTAL JOINT REINFORCEMENT SHALL CONSIST OF AT LEAST TWO WIRES OF W1.7 SPACED NOT MORE THAN 16 IN. HORIZONTAL REINFORCEMENT SHALL ALSO BE PROVIDED AT THE BOTTOM AND TOP OF WALL OPENINGS AND SHALL EXTEND NOT LESS THAN 24 IN. NOR LESS THAN 48 BAR DIAMETERS PAST THE OPENING; CONTINUOUSLY AT STRUCTURALLY CONNECTED ROOF AND FLOOR LEVELS; AND WITHIN 16 IN. OF THE TOP OF WALLS. DOWELS SHALL BE USED TO PROVIDE CONTINUITY INTO THE STRUCTURE ABOVE AND/OR BELOW, UNLESS NOTED OTHERWISE. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINE CONCRETE AND GROUT TO AREA REQUIRED.

WOOD

STRUCTURAL WOOD COMPONENTS (BEAMS, JOISTS, RAFTERS, SHEAR/BEARING WALLS, ETC.) SHALL BE CONSTRUCTED OF MINIMUM NO. 2 GRADE SOUTHERN PINE ACCORDING TO AF&PA NDS. WOOD IN CONTACT WITH CONCRETE OR MASONRY, AND AT OTHER LOCATIONS AS SHOWN ON THE DRAWINGS, SHALL BE PROTECTED OR PRESERVATIVE TREATED. MEMBER SIZES SHOWN ARE NOMINAL UNLESS NOTED OTHERWISE.

ENGINEERED WOOD TRUSS SYSTEMS SHALL BE DESIGNED BY SUPPLIER'S SPECIALTY ENGINEER TO CONFIGURATION AND LOAD—CARRYING CAPACITY SHOWN ON DRAWINGS AND SPECIFICATIONS. ALTERNATE TRUSS LAYOUTS ARE ACCEPTABLE ONLY AS A CHANGE ORDER WHICH WILL INCLUDE ENGINEERING CHARGES FOR REDESIGN OF THE STRUCTURE AND FOUNDATION BY THE ENGINEER OF RECORD. SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION AND BEFORE CONSTRUCTION OF THE FOUNDATION. SHOP DRAWINGS SHALL SHOW AND SPECIFY ALL CONNECTOR TYPES UTILIZED WITHIN TRUSSES, AS WELL AS CONNECTORS UTILIZED IN ALL OTHER CONNECTIONS AND ATTACHMENTS BETWEEN TRUSSES OR COMPONENTS SUPPLIED AS PART OF THE ENGINEERED TRUSS SYSTEM. AN ERECTION DRAWING SHALL BE INCLUDED, IDENTIFYING ALL TRUSS SYSTEM COMPONENTS, AS WELL AS ALL PERMANENT BRACING REQUIRED FOR TRUSS DESIGN. ENGINEER OF RECORD MUST REVIEW/ APPROVE ALL TRUSS LAYOUTS BEFORE FOUNDATION IN CONSTRUCTED.

ENGINEERED SHOP DRAWINGS SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER AS THE SPECIALTY ENGINEER. THE FOLLOWING LOAD DURATION FACTORS SHALL BE USED:

DEAD LOAD 0.90
DEAD LOAD + FLOOR LIVE LOAD 1.00
DEAD LOAD + ROOF LIVE LOAD 1.25
DEAD LOAD + WIND LOAD 1.33

CONNECTORS:

CONNECTOR MODEL NUMBERS SHOWN ARE Strong—tie CONNECTORS AS MANUFACTURED BY SIMPSON Strong—tie Company, U.N.O., AND SHALL BE INSTALLED ACCORDING TO THE MANUFACTURER'S DIRECTIONS TO RESIST THE SPECIFIED LOADS. ALL STRAPS SHALL BE GALVANIZED OR STAINLESS STEEL.

UPLIFT CONNECTIONS SHALL BE INSTALLED TO RESIST UPLIFT FORCES INDICATED BY THE SPECIALTY ENGINEER. AS FOLLOWS:

WOOD TO CONOD

WOOD TO CONCRETE

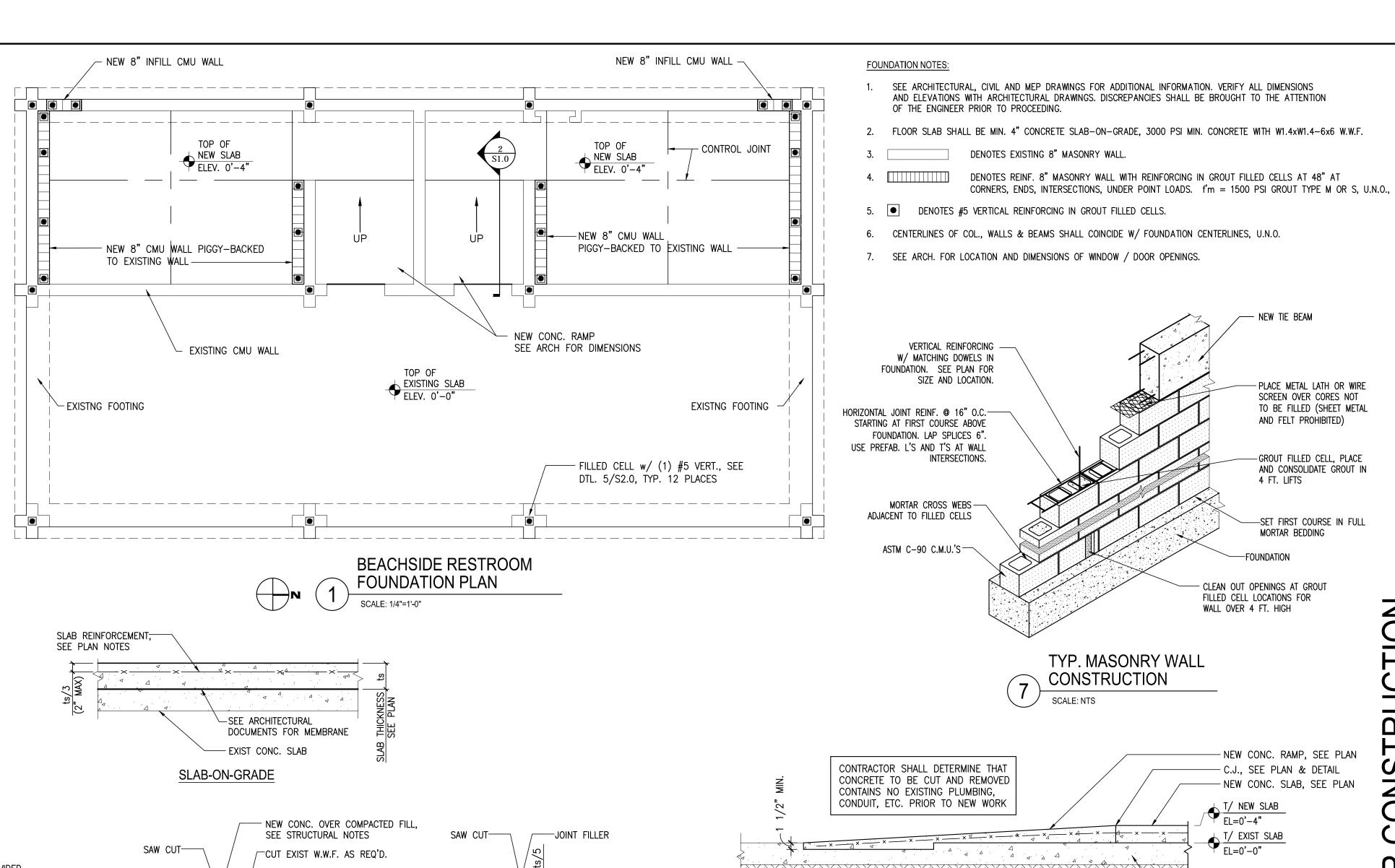
FOR UPLIFT LOADS UP TO 1450#, META 18

FOR UPLIFT LOADS OF 1450# TO 2900#, (2) META18
FOR UPLIFT LOADS OF 2900# TO 10530#, HGT-2/HGT-3

CONTRACTOR MAY USE APPROPRIATE SIMPSON CONNECTORS WITH LESS CAPACITY THAN SPECIFIED ABOVE WHEN LESS UPLIFT LOAD IS SHOWN ON THE INDIVIDUAL TRUSS DETAIL SHEETS (PROVIDED BY THE SPECIALTY ENGINEER)

| | SUPPLEMENTAL SKETCHES/ DRAWINGS:

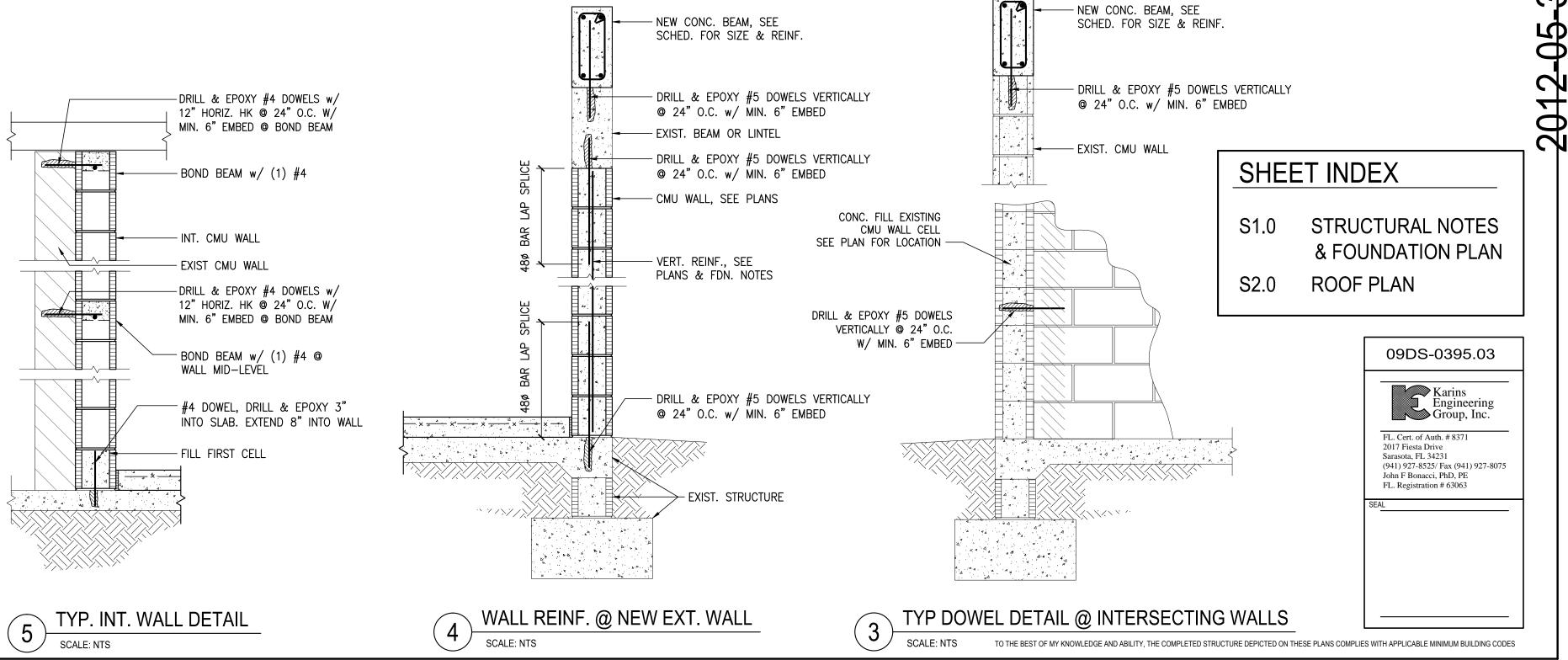
IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO FORWARD A COPY OF ALL CORRESPONDENCE AFFECTING THE STRUCTURE TO THE PROJECT'S INSPECTOR THROUGHOUT THE DURATION OF CONSTRUCTION.





CONTRACTION (CONTROL) JOINT

SLAB TRENCH FILI



∑ 2'−6" \

SCHENKELSHULTZ

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Coquina Beach

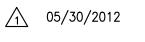
2650 Gulf Drive South Bradenton, Florida 34217

Beach Side Restroom Renovation

Manatee County Government

1112 Manatee Avenue West Bradenton, Florida 34208

revisions:



drawn: checked: date: 04.13.2012

date: 04.13.2012 comm. no.: 0920829

RESTROOM -STRUCTURAL NOTES & FOUNDATION PLAN

S1.0

permit drawings

GENERAL DEMOLITION NOTES:

- 2. CONTRACTOR SHALL INSPECT AND VERIFY THE SCOPE OF WORK, ANY ADDITIONAL WORK NOT SPECIFICALLY NOTED ON THE DRAWINGS BUT BECOMES APPARENT UPON CAREFUL FIELD INSPECTIONS SHALL BE PART OF THIS CONTRACT.
- 3. PROVIDE SMOOTH UNDETECTABLE TRANSITION FROM EXISTING TO NEW CONSTRUCTION.
- 4. OWNER HAS FIRST SALVAGE RIGHTS TO ANY ELEMENT OF EXISTING CONSTRUCTION TO BE REMOVED DURING DEMOLITION OPERATIONS.
- 5. PATCH AND REPAIR ALL CONSTRUCTION TO REMAIN WHERE AFFECTED BY DEMOLITION OPERATIONS.
- 6. PROTECT CONSTRUCTION TO REMAIN FROM WEATHER INTRUSION, VANDALISM AND ALL OTHER RISKS CREATED BY DEMOLITION OPERATIONS.
- 7. FIELD VERIFY ALL DIMENSIONS ASSOCIATED WITH EXISTING CONSTRUCTION.
- 8. WHERE DEMOLITION OPERATIONS EFFECT STRUCTURAL SYSTEM COMPONENTS TO REMAIN, SHORE THESE COMPONENTS AS REQUIRED UNTIL NEW CONSTRUCTION IS IN PLACE.
- 9. ALL EXISTING STRUCTURAL SUPPORT COMPONENTS SHALL REMAIN INTACT EXCEPT AS OTHERWISE INDICATED.
- 10. REMOVE OR CAP AND ABANDON ALL ELECTRICAL WIRING IN WALLS TO BE REMOVED.
- 11. REMOVE OR CAP FOR FUTURE USE ALL PLUMBING LINES AND OUTLETS IN WORK TO BE DEMOLISHED.
- 12. PATCH ANY EXISTING CONSTRUCTION AS REQUIRED WHICH IS DAMAGED DUE TO THE INSTALLATION OF NEW FIXTURES, ELECTRICAL DEVICES, EQUIPMENT OR ANY OTHER COMPONENT.
- 13. PROTECT ALL EXISTING FINISHES TO REMAIN FROM DAMAGE FOR DURATION OF NEW CONSTRUCTION.
- 14. PATCH AND REPAIR EXISTING CEILINGS WHERE ELECTRICAL AND MECHANICAL DEVICES HAVE BEEN REMOVED OR RELOCATED. MATCH EXISTING.
- 15. MODIFIED STRUCTURAL MEMBERS ARE IN COMPLIANCE WITH FLORIDA BUILDING CODE 2007 W/ REVISION 2009. MEMBERS NOT DESIGNATED FOR ALTERATIONS WERE ASSUMED ADEQUATE. NO EXHAUSTIVE STUDIES WERE PERFORMED BY KARINS ENGINEERING GROUP, INC. (KEG) PRIOR TO THIS DESIGN TO UNCOVER ALL CONSTRUCTION DEFECTS OR CODE VIOLATIONS.

- NEW CONC. BM.

EXIST. CMU COLUMN

(1) #5 HOOK BAR LAP w/ FILLED

@ LEVEL OF BEAM TOP BARS

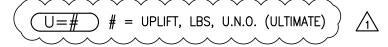
CÉLL BAR, PROVIDE STD. 900 HOOK

-(1) #5 IN CENTER OF FILLED CELL,

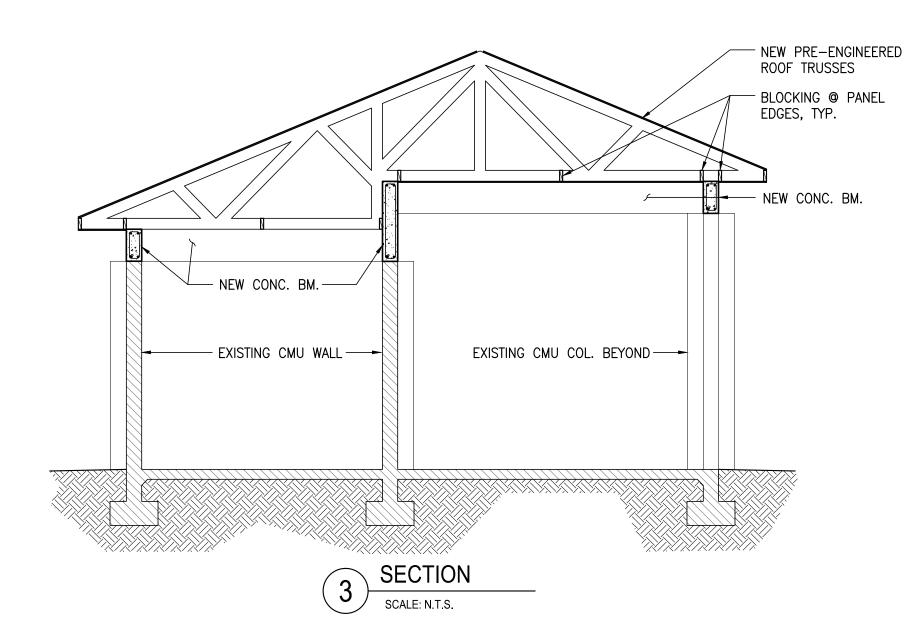
SÉÉ "PLAN FOR LOCATIONS

ROOF PLAN NOTES:

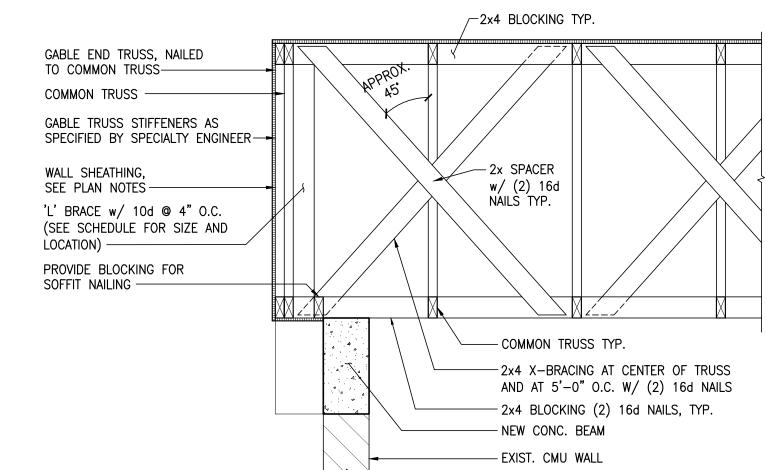
- ROOF SHALL CONSIST OF PRE-ENGINEERED WOOD TRUSSES @ 24" O.C. U.N.O. TOP CHORD SHALL BE MIN. 2x6. SEE ARCH. DRAWINGS FOR SLOPING AND TRUSS PROFILES.
- 2. ROOF DIAPHRAGMS SHALL CONSIST OF 19/32" CDX APA RATED SHEATHING WITH 10d @ 6" PANEL EDGES AND INTERMEDIATE AND 4" @ CORNERS. 2X BLOCKING SHALL BE INSTALLED BETWEEN TRUSS CHORDS AT EACH END BEARING AND WITHIN 6" OF EACH INTERIOR BEARING POINT.
- 3. PROVIDE 15/32" CDX APA RATED SHEATHING ON UNDERSIDE OF TRUSSES EXPOSED TO WIND, FASTEN WITH 10d NAILS @ 6" O.C.
- 4. EXTERIOR WALL TRUSS SHEATHING SHALL CONSIST OF 15/32" CDX PLYWOOD FASTENED WITH 10d NAILS AT 6/4 SPACING WITH ALL PANEL EDGES BLOCKED, U.N.O. BY SPECIALTY ENGINEER.
- 5. PROVIDE CONTINUOUS 2x4 BOTTOM CHORD BRACING @ 10'-0" O.C. PERPENDICULAR TO SPAN OF TRUSS.
- 6. SPECIALTY ENGINEER REFER TO ARCHITECTURAL FOR REFLECTED CEILING PLAN.
- 7. CONTRACTOR VERIFY ALL CONNECTOR SIZES WITH SPECIALTY ENGINEER TRUSS DRAWINGS

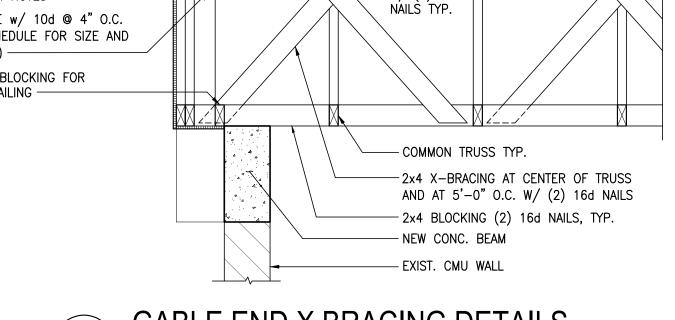


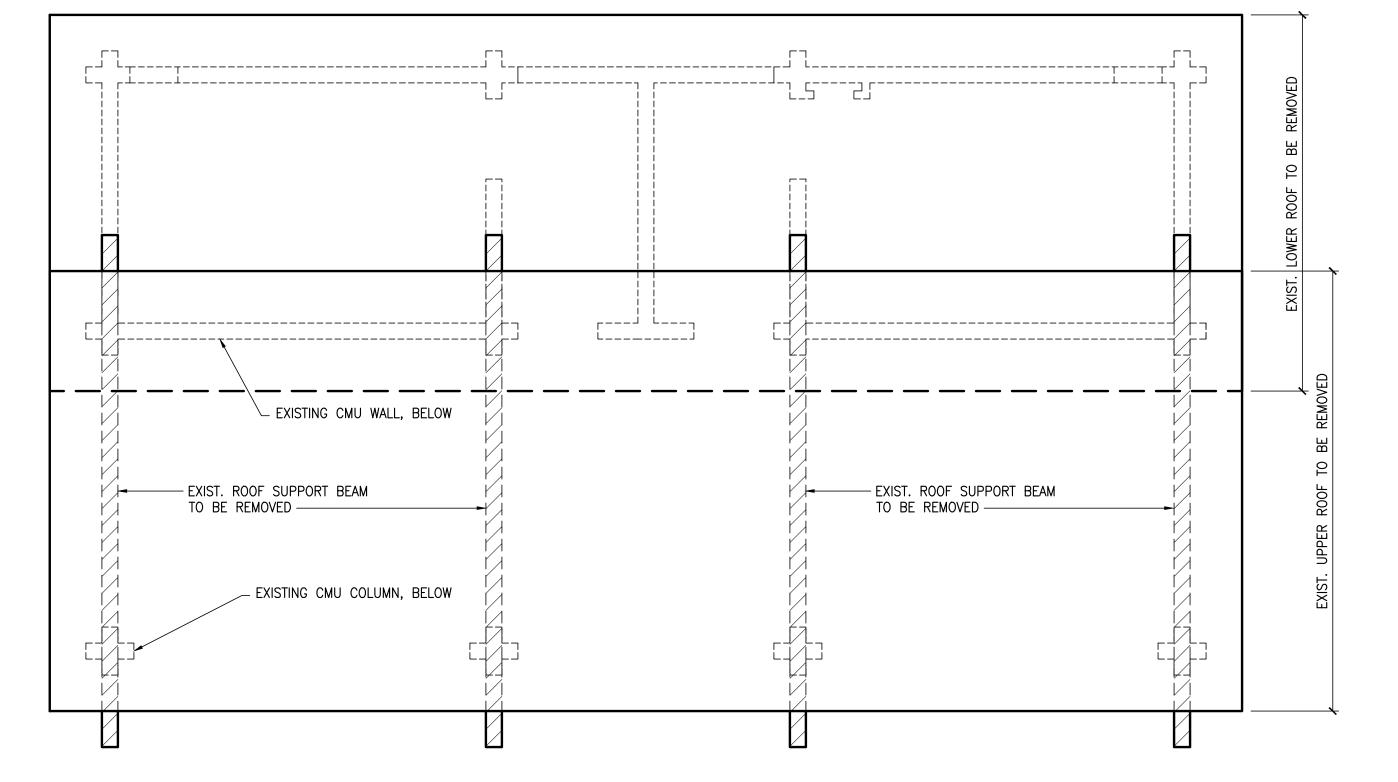
CONCRETE BEAM SCHEDULE								
MARK	SIZE WxD: (IN)	REINFORCEMENT						
		TOP	MIDDLE	воттом	STIRRUPS			
TB-1	8 x 16	(2) #5	_	(2) #5	#3@18" O.C			
TB-2	8 x 40	(2) #5	2x (2) #5	(2) #5	#3@18" O.C			
B-1	8 x 16	(2) #5	_	(2) #5	#3@7" O.C.			



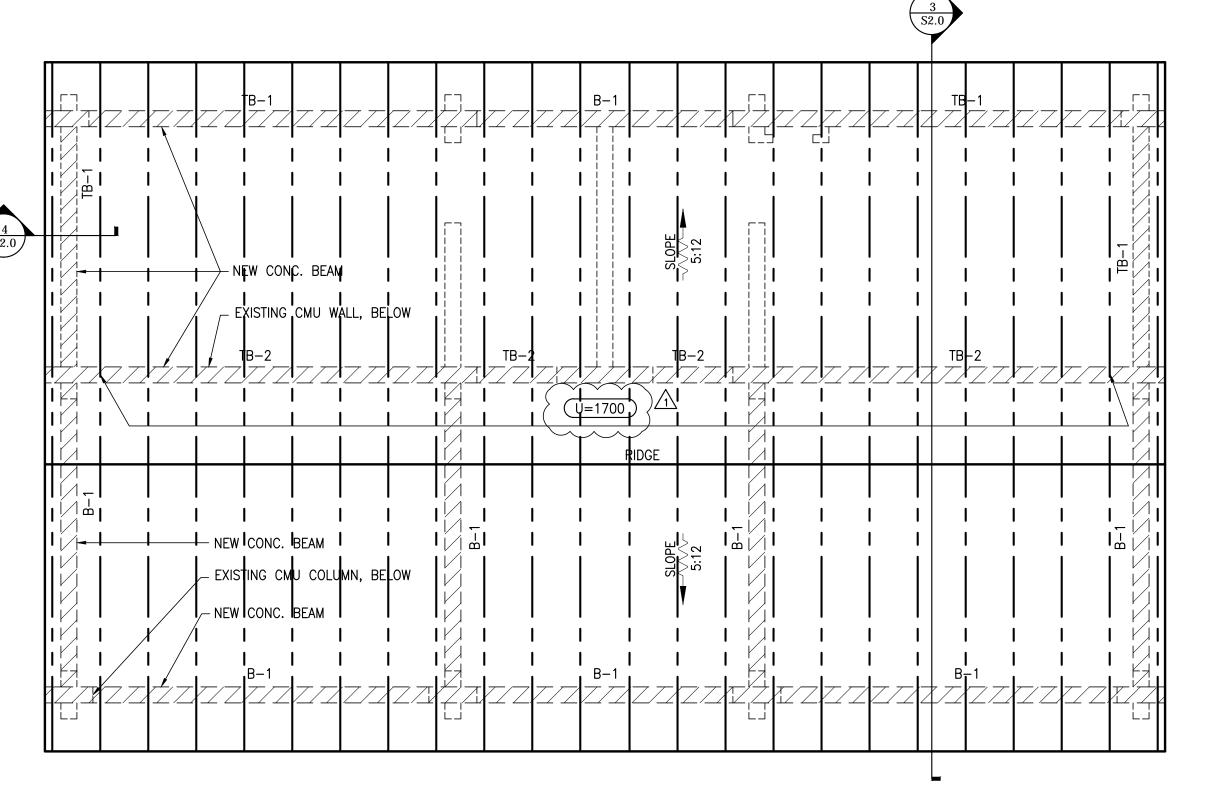
'L' BRACING REQUIREMENTS								
STUD SPACING	NO 'L' BRACE	(1) 2" x 4" 'L' BRACE	(2) 2" x 4" 'L' BRACE	(2) 2" x 6" 'L' BRACE				
16"	0'-0" - 4'-5"	4'-5" - 8'-0"	8'-0" - 12'-0"	12'-0" - 16'-0"				
24"	0'-0" - 4'-0"	4'-0" - 7'-0"	7'-0" - 10'-0"	10'-0" - 14'-0"				
_	SPECIALTY ENGINEER VERIFY							











BEACHSIDE RESTROOM NEW ROOF PLAN SCALE: 1/4"=1'-0"

09DS-0395.03 Karins Engineering Group, Inc. FL. Cert. of Auth. # 8371 2017 Fiesta Drive Sarasota, FL 34231 (941) 927-8525/ Fax (941) 927-8075 John F Bonacci, PhD, PE FL. Registration # 63063

30

201

RESTROOM -**ROOF PLAN**

BEAM BEARING @ CMU SUPPORTS NOT TO SCALE

48 LAP



GABLE END X BRACING DETAILS

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH APPLICABLE MINIMUM BUILDING CODES

Coquina Beach 2650 Gulf Drive South Bradenton, Florida 34217 Beach Side Restroom Renovation

SCHENKELSHULTZ

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05/30/2012

drawn: checked: date: 04.13.2012

comm. no.: 0920829

permit drawings