June 30, 2023

Kimley-Horn and Associates, Inc. 201 North Franklin St. Suite 1400 Tampa, FL 33602

Attn: Ms. Shari Barnwell, P.E.

RE: Roadway Soil Survey Report Phase III Plans Submittal Lena Road from North of 44<sup>th</sup> Avenue East to SR 64 Manatee County, Florida Manatee County Project #6107560 Kimley-Horn Project No.: 148400100 Tierra Project No. 6511-22-127

Ms. Barnwell:

Tierra, Inc. (Tierra) has completed a Roadway Soil Survey Report for the above referenced project. This report is being provided to support the Phase III plans submittal. The results of our field exploration program and laboratory testing performed to date and subsequent geotechnical recommendations are presented herein.

Tierra, Inc. appreciates the opportunity to be of service to Kimley-Horn and Associates, Inc. (KHA) on this project. If you have any questions or comments regarding this report, please contact our office at your earliest convenience.

Sincerely,

TIERRA, INC.

un Vinco

Trevor J. Bianco, E.I. Geotechnical Engineer Intern

Kevin H. Scott, P.E. Senior Geotechnical Engineer Florida License No. 65514

# Table of Contents Page 1 of 2

1.0	PROJECT INFORMATION	1
1.1	Project Authorization	.1
1.2	Project Description	.1
1.3	General Site Conditions	.1
2.0	PURPOSE AND SCOPE OF SERVICES	1
3.0	REVIEW OF PUBLISHED DATA	2
3.1	Regional Geology	.2
3.2	USDA Soil Survey	.3
3.3	USGS Quadrangle Maps	.3
3.4	Potentiometric Surface Elevation	.3
10		1
<b>4.0</b>	Piezometers	5
4.1	Bulk Sampling and LBR Testing	.5 5
4.2	Double Ring Infiltration Tests (DRIT)	.5 5
4.0		.0
50	LABORATORY TESTING	5
51	General	5
5.2	Test Designation	.5
0.2		.0
60		6
6.1	Conoral Soil Conditions	6
6.2	Groupdwater	.0
63	Seesonal High Groundwater Estimates	.1 7
6.4	Organic Soils	. <i>1</i> 7
0. <del>4</del> 6.5	Landfill Debris	י. א
0.0		.0
70	ENGINEERING EVALUATIONS AND RECOMMENDATIONS	Q
7.0	General	8
7.1	Landfill Debris	ט. ג
73	Embankment Settlement	ט. א
7.0	Slone Stability	.υ α
7.5	Temporary Slopes and Trenches	 Q
7.6	Groundwater Control	.5 م
7.5	Existing Pavement Conditions	.0 .0
78	Pavement Design Considerations	.0 9
79	On-Site Soil Suitability	10
7 10	General Roadway Construction	10
7.10		
<u>ه</u> ۸		1
0.0		

# Table of Contents Page 2 of 2

#### APPENDIX A

USDA Soil Survey and USGS Quadrangle Maps Summary of USDA Soil Survey Information

#### **APPENDIX B**

Roadway Soil Survey Boring Location Plan Roadway Soil Profiles Pond Soil Survey Debris Soil Profiles Debris Delineation Plan Muck Delineation Plan

#### APPENDIX C

Summary of Seasonal High Groundwater Table Estimates for Roadway Summary of Seasonal High Groundwater Table Estimates for Ponds Summary of Groundwater Table Measurements from Piezometers

#### APPENDIX D

Summary of Laboratory Test Results for Soil Classification Summary of Laboratory Test Results for Environmental Classification Hydraulic Conductivity Test Results Design LBR and MR Summary Table Results of Limerock Bearing Ratio Tests

#### APPENDIX E

Pavement Data and Condition Sheet Pavement Core Photographs

#### **APPENDIX F**

Test Pit Sample Photographs

Roadway Soil Survey Report Lena Road from North of 44<sup>th</sup> Avenue East to SR 64 Manatee County, Florida Tierra Project No. 6511-22-127 Page 1 of 11

# 1.0 **PROJECT INFORMATION**

#### 1.1 **Project Authorization**

Authorization to proceed with this project was issued by KHA in accordance with the Subconsultant Agreement for the referenced project.

#### 1.2 **Project Description**

The project, as we understand it, consists of preparing construction plans for proposed roadway improvements consisting of 84 feet of right of way containing a two-lane roadway with 11-foot travel lanes, an 18-foot wide median or a 14-foot two way left turn lane, a 12-foot shared use path on the west side of the road and a 5-foot sidewalk on the east side of the road in Manatee County, Florida.

The purpose of this report is to provide geotechnical (i.e. soils and groundwater) input to the design team to assist in design of the proposed roadway improvements. This report concentrates on the roadway and drainage portions of the project and is provided to be included with the Phase III plans submittal.

#### **1.3 General Site Conditions**

The existing typical section along Lena Road is a 2-lane roadway with 12-foot travel lanes and unpaved shoulders. The southern portion of Lena Road provides a 5-foot sidewalk on the east side of the roadway and a 10-foot sidewalk outside of the roadway right of way on the west side. The right of way varies throughout the project limits with typical widths of 90-feet, 50-feet, and finally widening to 96-feet approaching SR 64, starting from the southern end. The southern portion of the proposed Lena Road alignment from Station 218+20 to approximately 241+75 consists of undeveloped wooded land and two (2) existing stormwater ponds. The proposed alignment traverses across these existing pond areas.

# 2.0 PURPOSE AND SCOPE OF SERVICES

This geotechnical study was performed to obtain information on the existing subsurface conditions along the limits of the proposed roadway, FPC and stormwater improvements to assist in design of the construction plans for the project. The following services were provided:

- Reviewed published soil information obtained from the "Soil Survey of Manatee County, Florida" published by the United States Department of Agriculture (USDA) Natural Resources Conservation Services (NRCS). Reviewed topographic data obtained from the "Lorraine, Florida" Quadrangle Map. Reviewed potentiometric surface elevations obtained from the "Potentiometric Surface of the Upper Floridan Aquifer" maps published by the USGS.
- 2. Conducted a visual reconnaissance of the project site and coordinated utility clearances via Sunshine State One Call.

- 3. Performed a geotechnical field study to evaluate the existing subsurface conditions along the project alignments consisting of borings, pavement cores, subsurface sampling and field-testing.
- 4. Performed test pits in Parcel 103 to explore the depths and boundaries of the encountered landfill debris.
- 5. Installed 11 piezometers to monitor groundwater levels to assist in estimating SHGWT levels.
- Coordinated with the project surveyor to provide survey data (location and elevation) for the piezometers installed and borings performed along the project alignment and with the stormwater ponds where the Seasonal High Groundwater Table (SHGWT) was estimated.
- 7. Visually classified and stratified recovered soil samples in the laboratory. Performed laboratory tests on selected representative samples to develop the soil legend for the project in accordance with the American Association of State Highway and Transportation Officials (AASHTO) soil classification system.
- 8. Prepared this Roadway Soil Survey Report for the project.

# 3.0 REVIEW OF PUBLISHED DATA

#### 3.1 Regional Geology

The following paragraphs on the geology of Manatee County were paraphrased from the Florida Geological Survey, Open-File Report 80, 2001 and other geologic references.

The near surface geologic deposits and formations from youngest to oldest in Manatee County include: Undifferentiated sediments (Qu, TQu), Shelly sediments (TQsu), the Hawthorn Group Peace River Formation Bone Valley Member (Thpb), the Hawthorn Group Arcadia Formation (Tha), and the Hawthorn Group Arcadia Formation Tampa Member (That).

The Undifferentiated sediments and Beach and Ridge dunes are siliciclastics that are light gray, tan or brown to black in color, unconsolidated to poorly consolidated, clean to clayey silty, unfossiliferous, variably organic-bearing sands to blue green to olive green, poorly to moderately consolidated, sandy, silty clays. The Shelly sediments are variably calcareous and fossiliferous quartz sands to well indurated, sandy, fossiliferous limestones with clayey sands and sandy clays.

The Peace River Formation is primarily found near the surface in northwestern Manatee County and is composed of interbedded sands, clays and carbonates. The sands are generally light gray to olive gray in hue, poorly consolidated, clayey, variably dolomitic, very fine to medium grained and phosphatic. The clays are yellowish gray to olive gray in color, poorly to moderately consolidated sandy, silty, phosphatic and dolomitic. The carbonates are light gray to yellowish gray, poorly to well indurated, variably sandy and clayey, and phosphatic. The carbonates often include opaline chert. The Bone Valley Member is a clastic unit consisting of sand-sized and larger phosphate grains in a matrix of quartz sand, silt and clay. The lithology is highly variable ranging from sandy, silty, phosphatic clays and relatively pure clays to clayey, phosphatic sand to sandy, clayey phosphorites and is found within 50 feet of the surface in eastern Manatee County. The Peace River Formation is a semi-confining unit and forms an intermediate aquifer system in eastern Manatee County.

The Arcadia Formation is predominantly a carbonate unit with a variable siliciclastic component. The Arcadia Formation is composed of yellowish gray to light olive gray to light brown, micro to finely crystalline, variably sandy, clayey and phosphatic, fossiliferous limestones and dolostones. Thin beds of sand and clay are common. The sands are yellowish gray, very fine to medium grained, poorly to moderately indurated, clayey, dolomitic and phosphatic. The clays are yellowish gray to light olive gray in hue, poorly to moderately indurated, sandy, silty, phosphatic and dolomitic.

The Tampa member of the Arcadia Formation is white to yellowish gray in color, fossiliferous and variably sandy and clayey mudstones, wackestone and packstone with minor to no phosphate grains. In Manatee County, the Tampa member is found about 300 feet below land surface (bls), is approximately 100 to 150 feet thick and is part of the Floridan Aquifer System.

#### 3.2 USDA Soil Survey

Based on a review of the Manatee County Soil Survey published by the USDA, it appears that there are eight (8) primary soil-mapping units noted along the project alignment and within the proposed pond sites. An illustration of the **USDA Soil Survey Map** is provided in **Appendix A** and a summary of each soil unit is provided in **Appendix A**.

It should be noted that information contained in the USDA Soil Survey may not be reflective of actual soil and groundwater conditions, particularly if recent development in the project vicinity has modified soil conditions or surface/subsurface drainage.

#### 3.3 USGS Quadrangle Maps

Based on a review of the USGS Quadrangle Map titled "Lorraine, Florida", it appears that the project site natural elevations range from approximately +20 feet to +35 feet National Geodetic Vertical Datum of 1929 (NGVD 29) as illustrated on the **USGS Quadrangle Map** provided in **Appendix A**.

#### 3.4 Potentiometric Surface Elevation

Based on a review of the "Potentiometric Surface of the Upper Floridan Aquifer" maps published by the USGS; the potentiometric surface elevation of the upper Floridan Aquifer in the project vicinity ranges from approximately +20 to +30 feet, NGVD 1929.

As indicated in Section 3.3, the project site natural ground elevations range from approximately +20 to +35 feet NGVD 1929. Artesian conditions were not encountered at the time of our field activities; however, the Contractor should be prepared to address artesian levels up to a head of +30 feet NGVD 1929.

# 4.0 SUBSURFACE EXPLORATION

Prior to commencing our subsurface explorations, boring location plans for the proposed roadway alignment were produced. The boring location plans were generated based on a review of the project design files provided by KHA, general guidance provided in the FDOT "Soils and Foundations Handbook" and our engineering judgment.

To evaluate the subsurface conditions and groundwater table levels, over 250 hand auger borings were advanced to depths ranging from approximately 1½ to 8 feet below the existing ground surface and over 40 Standard Penetration Test (SPT) borings were performed to depths of 10 to 50 feet below the existing ground surface along the project alignments and within the proposed stormwater ponds and FPC sites. A total of twelve (12) pavement cores were performed within the existing lanes of Lena Road to identify the existing pavement section. In addition, a series of test pits were performed using a mechanical excavator to further explore the encountered landfill material in Parcel 103.

The hand auger borings were performed by manually twisting and advancing a bucket auger into the ground, typically in 6-inch increments. As each soil type was revealed, representative samples were collected and returned to our office for confirmation of the field classification by a geotechnical engineer. Hand auger borings were performed to depths ranging from 1 foot to 5 feet below the existing pavement surface at each core location to evaluate the subgrade material. Following completion of the borings, the auger holes were backfilled with sand and compacted. The pavement cores were then patched with an FDOT approved pavement-patching product.

The SPT borings were performed using mechanical and barge mounting drilling equipment utilizing bentonite mud drilling procedures. The soil sampling was performed in general accordance with the American Society for Testing and Materials (ASTM) test designation D-1586. In general, the SPT borings were advanced by hand auger from the ground surface to depths of 4 and 6 feet to verify utility clearance. SPT resistance N values were then taken continuously to a depth of 10 feet and on intervals of 5 feet thereafter to the boring termination depth.

The pavement cores were performed with the use of a 4-inch outside diameter core bit. The asphalt pavement was visually classified using standard FDOT nomenclature. Core samples of the existing pavement section were collected and the thickness of the pavement and base was measured. In addition, observed cracking, rut depths, general pavement condition, subgrade material and measured cross slopes are presented along with the pavement core data on the **Pavement Data and Condition** sheet in **Appendix E**. Photographs of the asphalt pavement cores are included in the **Pavement Core Photographs** sheets in **Appendix E**.

The locations and ground surface elevations of the majority of the borings performed for evaluation of the SHGWT were determined by the project surveyor. The locations of the remainder of the borings and cores were estimated using the GPS coordinates obtained in the field by representatives of Tierra using hand-held, non-survey grade Garmin eTrex® Global Positioning System (GPS) devices with a manufacturer's reported accuracy of ±10 feet and therefore should be considered approximate.

Roadway Soil Survey Report Lena Road from North of 44<sup>th</sup> Avenue East to SR 64 Manatee County, Florida Tierra Project No. 6511-22-127 Page 5 of 11

The locations of the borings and cores performed for this study are shown on the **Boring Location Plan** sheets in **Appendix B**. The Station and Offset of each boring are labeled on the **Roadway Soil Profiles** and **Pond Soil Survey** sheets in **Appendix B**. The approximate Station and Offset and State Plane West coordinates of the core locations are presented on the **Pavement Data and Condition Sheet** table in **Appendix E**.

#### 4.1 Piezometers

Tierra installed eleven (11) piezometers to depths of approximately 5½ feet below the existing ground surface along the project corridor. The purpose of the piezometers was to monitor groundwater levels to assist in estimating SHGWT levels. The groundwater table levels within the piezometers were monitored and recorded three times between July and November of 2022. A summary table of the recorded water levels in the piezometers to date is included in **Appendix D**.

#### 4.2 Bulk Sampling and LBR Testing

Bulk samples for Limerock Bearing Ratio (LBR) testing were retrieved at fourteen (14) locations along the proposed roadway alignment and within the pond areas. In general, these samples were collected within the top 2 feet of the near-surface soils encountered. The results of the LBR tests are presented in **Appendix C**.

#### 4.3 Double Ring Infiltration Tests (DRIT)

One (1) Double Ring Infiltration Test (DRIT) was performed within a proposed stormwater pond area. The test was performed at a depth of 1 foot below grade. The test location is depicted on the **Boring Location Plan** sheets and the results are presented in the **Hydraulic Conductivity Test Results** table in **Appendix C**.

# 5.0 LABORATORY TESTING

#### 5.1 General

Representative soil samples collected from the borings performed along the project alignments were classified and stratified in general accordance with the AASHTO soil classification system. Our classification was based on visual observations, using the results from the laboratory testing as confirmation. These tests included grain-size analyses, fines content, organic content, Atterberg limits and natural moisture content determination. In addition, environmental corrosion tests were performed on selected soil samples to evaluate the corrosive nature of the subsurface soils encountered.

#### 5.2 Test Designation

The following list summarizes the laboratory tests performed by Tierra and the respective test methods utilized.

• <u>Fines Content Analyses</u> - The fines content tests were conducted in general accordance with the AASHTO test designation T-088 (ASTM test designation D-1140).

- <u>Grain-Size Analyses</u> The grain-size analyses were conducted in general accordance with the AASHTO test designation T-088 (ASTM test designation D-422).
- <u>Atterberg Limits</u> The liquid limit and the plastic limit tests ("Atterberg Limits") were conducted in general accordance with the AASHTO test designations T-089 and T-090, respectively (ASTM test designation D-4318).
- <u>Organic Content</u> The organic content tests were conducted in general accordance with the AASHTO test designation T-267.
- <u>Natural Moisture Content</u> The moisture content tests were conducted in general accordance with the AASHTO test designation T-265 (ASTM test designation D-2216).
- <u>Environmental Corrosion</u> The environmental corrosion tests were conducted in general accordance with the FDOT test designations FM 5-550, FM 5-551, FM 5-552 and FM 5-553.

A summary of the laboratory test results for each soil stratum encountered along the project alignments and pond sites is presented on the **Roadway Soil Survey** sheet in **Appendix B**. These sheets include ranges of laboratory test results for different stratum soil samples collected from borings performed along the project alignment. A detailed summary of the laboratory test results performed for this report is presented in **Appendix D**.

# 6.0 **RESULTS OF SUBSURFACE EXPLORATION**

#### 6.1 General Soil Conditions

The soil types encountered during this exploration have been assigned a stratum number. The stratum number and soil types associated with the roadway portion of this project are provided below:

Stratum Number	Typical Soil Description	AASHTO Classification
1	Brown to Light Brown to Gray to Light Gray Sand to Sand with Silt	A-3
2	Brown to Light Brown to Gray to Light Gray Silty Sand	A-2-4
3	Brown to Light Brown to Gray Silty Sand to Silty-Clayey Sand	A-2-4
4	Gray to Brown Clayey Sand to Sandy Clay	A-2-6/A-4/A-6
5	Dark Gray to Black Organic Sand to Organic Silty Sand to Peat	A-8
6	Gray to Brown Silt to Clay	A-7-6/A-7-5
7	Calcareous Clay to Weathered Limestone	(1)
8	Landfill Debris	
<sup>(1)</sup> USCS d	loes not include nomenclature for Limestone	

A geotechnical engineer bases soil stratification on a visual review of the recovered samples, laboratory testing and interpretation of the field boring logs. The boring stratification lines represent the approximate boundaries between soil types of significantly different engineering properties; however, the actual transition may be gradual. In some cases, small variations in properties within the same boring not considered pertinent to our engineering evaluation may have been abbreviated or omitted for clarity. The boring profiles represent the conditions at the particular boring location and variations do occur among the borings.

The results of the borings performed for this project along with the boring location plans are presented in **Appendix B** of this report.

#### 6.2 Groundwater

The groundwater table was recorded at each of the boring locations during our field exploration. The depths to the groundwater table along the project alignments were found to range from approximately at or above natural grade to 6½ feet below the existing ground surface at the locations of the borings performed. The groundwater table measured at each of the boring locations is presented on the **Roadway Soil Profiles** sheets in **Appendix B**. The groundwater table was not encountered within some borings performed prior to termination of the boring. As a result, GNE (Groundwater Not Encountered) is indicated on those soil profiles on the **Roadway Soil Profiles** sheets. Likewise, the groundwater table was not recorded in the test pits and muck hand auger borings. As a result, GNR (Groundwater Not Recorded) is indicated on those soil profiles.

Groundwater conditions will vary with environmental variations and seasonal conditions, such as the frequency and magnitude of rainfall patterns, as well as man-made influences (i.e., existing water management canals, swales, drainage ponds, underdrains, and areas of covered soils, such as paved parking lots and sidewalks).

#### 6.3 Seasonal High Groundwater Estimates

Tierra performed hand auger borings at selected locations along the project alignment and within the ponds sites to estimate the SHGWT. The SHGWT levels at these boring locations were estimated based on a review of the soil samples, natural indicators within the soils such as stain lines/mottles, measured groundwater levels in the borings, and the USDA Manatee County Soil Survey information. A summary of the SHGWT estimates are presented in **Appendix C**.

#### 6.4 Organic Soils

Organic sandy/silty soils (Stratum 5, A-8) were encountered during our roadway soil survey along portions of the project roadway alignment. The organic soils were encountered at depths ranging from approximately at grade to 5 feet below the existing ground surface. It is recommended that these soils be removed in accordance with FDOT Standard Plans Index 120-002 and utilized in accordance with FDOT Standard Plans Index 120-001. The borings that encountered organics are presented on the **Muck Delineation Plan** sheets in **Appendix B**. Tierra has coordinated with KHA to include hatching on the roadway cross-sections identifying the approximate limits of organic soil removal prior to the next submittal.

#### 6.5 Landfill Debris

Landfill debris consisting of glass, metal, plastic, wood and petroleum-like products were encountered along the proposed roadway alignment from approximately Station 233+50 to 244+00 and elsewhere within Parcel 103. These materials were encountered to depths exceeding 12 feet below existing grade. Soil borings and test pits were performed in these areas to delineate the approximate extents of the landfill debris. The locations and depths of the encountered landfill debris are shown on the **Debris Delineation Plan** sheet in **Appendix B**. Representative photographs of the encountered debris material and a historical photograph of the approximate limits of the landfill are included in **Appendix F**. If encountered, the debris materials should be removed and disposed of offsite.

# 7.0 ENGINEERING EVALUATIONS AND RECOMMENDATIONS

#### 7.1 General

In general, the existing subsurface soils encountered in the borings performed along the project alignment are suitable for supporting the proposed roadway improvements after proper subgrade preparation.

All earthwork activities including the site preparation, clearing and grubbing, removal and utilization/placement of soils, compaction of subgrade soils and selection of backfill materials should be accomplished in accordance with the current County Specifications and/or FDOT Standards and Specifications.

### 7.2 Landfill Debris

In Parcel 103, debris material (Stratum 8) was encountered within many borings beginning at grade and extending to a depth beyond 12 feet below grade. The encountered debris consisted of glass, metal, plastic, wood and petroleum-like products. **Debris can cause differential and/or excessive settlement in the roadway alignment and proposed mitigation area.** If encountered, these materials shall be removed and not used within the project limits and disposed of offsite. The boring soil profiles performed in Parcel 103 are presented on **Debris Soil Profiles** sheets in **Appendix B**. Additionally, the borings that encountered Strata 8 materials is presented on the **Debris Delineation Plan** sheets in **Appendix B**. Representative photographs of the encountered debris along with a historical photograph showing the affected area of Parcel 103 are included in **Appendix F**.

#### 7.3 Embankment Settlement

Embankment fill soils should be placed and compacted in accordance with the project specifications. For the anticipated embankment construction, we expect total settlements on the order of 2 inches or less, and differential settlements on the order of half the total settlements. The total and differential settlements are expected to occur predominately during construction.

### 7.4 Slope Stability

The embankment side slopes for the proposed roadway widening are minor. Based on soil conditions encountered throughout the site and based on our engineering judgement, the slopes will achieve factors of safety exceeding the minimum required safety factor of 1.3. As a result, Tierra does not anticipate limitations to the proposed roadway performance with the proposed embankments sloped provided that the embankments are constructed in accordance with current County Specifications and/or FDOT Specifications.

#### 7.5 Temporary Slopes and Trenches

Temporary side slopes and excavations should comply with the Occupational Safety and Health Administration's (OSHA) trench safety standards, 29 C.F.R., s. 1926.650, Subpart P, all subsequent revisions or updates of OSHA's referenced standard adopted by the Department of Labor and Employment Security and Florida's Trench Safety Act, Section 553.62, Florida Statutes. Excavated materials should not be stockpiled at the top of the slope within a horizontal distance equal to the excavation depth.

#### 7.6 Groundwater Control

The groundwater levels presented in this report are the levels that were measured at the time of our field activities. Fluctuation should be anticipated. Tierra recommends that the Contractor determine the actual groundwater levels at the time of the construction to determine groundwater impacts on the planned construction procedure.

#### 7.7 Existing Pavement Conditions

To evaluate the existing pavement conditions along the project alignment, Tierra performed twelve (12) pavement cores. Observed cracking, rut depths, general pavement condition, subgrade material and measured cross slopes are presented along with the pavement core data on the **Pavement Data and Condition** sheet in **Appendix E**. Photographs of the asphalt pavement cores are included in the **Pavement Core Photographs** sheets in **Appendix E**.

The pavement design engineer should review the current FDOT Flexible Pavement Design Manual for roadway projects to ensure an adequate Structural Number (SN) value is obtained based on the corresponding ESAL value over the design life of the roadway improvements. In addition, due to cracking of the structural asphalt layer at some of the pavement core locations along the project alignment, layer reduction coefficients for the existing pavement section should be reviewed and applied as necessary.

#### 7.8 Pavement Design Considerations

As previously mentioned, bulk samples were collected and LBR tests were performed by Tierra on the soil samples obtained along the project alignment. The Design LBR value was obtained by applying the  $\pm 2\%$  of Optimum Method and 90% Method in accordance with the FDOT Soils and Foundations Handbook. The design LBR value was chosen by taking the lower of the two LBR values determined by the two methods per FDOT Soils and Foundations Handbook.

Based on the LBR test results and the FDOT Flexible Pavement Design Manual, a design  $M_R$  value of 9,750 psi is recommended for use in pavement design for project roadway. It should be noted that the design  $M_R$  value obtained from the tests performed may not be representative of borrow materials which may support some of the proposed roadway.

In accordance with FDOT guidelines, grades for this type of roadway should be ideally set to provide a minimum separation per FDOT, PPM between the bottom of the base and the estimated seasonal high groundwater levels. Correspondingly, the base should remain equally above sustained water treatment levels in roadside ditches, making positive drainage of the ditches important. The choice of base material would depend upon the relationship of final roadway improvement grades and the bottom of the base to the estimated seasonal high groundwater table levels.

#### 7.9 On-Site Soil Suitability

The general suitability and evaluations of the soils encountered during our geotechnical exploration is presented on the **Roadway Soil Survey** sheet in **Appendix B**. FDOT Standard Plans, Indices 120-001 and 120-002 of the Design Standards should be consulted to determine the specific use/suitability of the soil types present within the project limits.

#### 7.10 General Roadway Construction

The overall site preparation and mechanical densification work for the construction of the proposed roadway should be in accordance with the current County Specifications and/or FDOT Specifications.

Roadway Soil Survey Report Lena Road from North of 44<sup>th</sup> Avenue East to SR 64 Manatee County, Florida Tierra Project No. 6511-22-127 Page 11 of 11

# 8.0 **REPORT LIMITATIONS**

Our services have been performed, our findings obtained, and our recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices at the time of this report. Our geotechnical engineering evaluation of the site and subsurface conditions with respect to the planned roadway construction, and our recommendations are based upon the following: (1) site observations, (2) the field exploratory test data obtained during the geotechnical study, and (3) our understanding of the project information and anticipated grades as presented in this report. This company is not responsible for the conclusions, opinions or recommendations made by others based on these data.

The scope of the exploration was intended to evaluate soil conditions within the influence of the proposed roadway construction. The analyses and recommendations submitted in this report are based upon the anticipated location and type of construction and data obtained from the soil borings performed at the locations indicated and does not reflect any variations which may occur among these borings. If any variations become evident during the course of construction, a re-evaluation of the recommendations contained in this report will be necessary after we have had an opportunity to observe the characteristics of the conditions encountered.

The scope of services, included herein, did not include any environmental assessment for the presence or absence of hazardous or toxic materials in the soil, surface water, groundwater, or air, on the site, below, and around the site. Any statements in this report or on the boring logs regarding odors, colors, unusual or suspicious items and conditions are strictly for the information of Kimley-Horn and Associates, Inc. and the Manatee County.

# **APPENDIX A**

USDA Soil Survey and USGS Quadrangle Maps

Summary of USDA Soil Survey Information



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	Summary of USDA Soil Survey Manatee County, Florida							
		N	Ianatee County	y, Florida				
USDA Map Symbol		Soil C	lassification			Seasonal Hi	gh Water Table	
and Soil Name	Depth (in)	USCS	AASHTO	Permeability (in/hr)	рн	Depth (feet)	Months	
	0-8	PT	A-8	6.0 - 20.0	3.5-6.0			
	8-24	SP, SP-SM	A-3	6.0 - 20.0	6.1-8.4	+2.0 - 0.0	Jan - Dec	
(7)	24-68	SC, SC-SM, SM	A-2-4, A-3	0.6 - 6.0	7.4-8.4			
Canova	0-16	SM, SP-SM	A-2-4, A-3	6.0 - 20.0	5.6-8.4			
Okeelanta	16-80	SM, SP, SP-SM	A-2-4, A-3	6.0 - 20.0	5.6-8.4	+1.0 - 0.0	Jun - Dec	
Choolanta	0-20	PT	A-8	6.0 - 20.0	4.5-6.5		Jan	
	20-54	SM, SP, SP-SM	A-2-4, A-3	6.0 - 20.0	5.1-7.8	+1.0 - 0.0	Jun - Dec	
	0-5	SM, SP-SM	A-2-4, A-3	6.0 - 20.0	3.5-6.5			
(11) Cassia Fine Sand, 0 to 2 percent slopes	5-26	SM, SP-SM	A-3, A-2-4	6.0 - 20.0	3.5-6.0	15 25	hup Nov	
	26-42	SM, SP-SM	A-2-4	0.6 - 6.0	3.5-5.5	1.5 - 3.5	Juli - Nov	
	42-80	SP-SM, SM	A-2-4, A-3	6.0 - 20.0	3.5-6.5			
(1.5)	0-5	SP, SP-SM	A-3	20.0 - 50.0	4.5-6.0			
(12)	5-29	SP, SP-SM	A-3	6.0 - 20.0	4.5-6.0		Jan	
Cassia Fine Sand,	29-41	SP, SP-SM	A-2-4, A-3	2.0 - 6.0	4.5-6.0	3.5 - 5.0	Jul - Dec	
	41-80	SP, SP-SM	A-3	20.0 - 50.0	4.5-6.0			
(1.5)	0-15	SC-SM, SM, SP-SM	A-3, A-2-4	6.0 - 20.0	5.6-7.3			
(16) Delrey Compley	15-55	SP-SM	A-3, A-2-4	6.0 - 20.0	5.6-7.3	0.0 - 0.5	Jan - Mar	
Deiray Complex	55-80	SC, SC-SM, SM	A-2-6, A-2-4	0.6 - 6.0	6.6-7.8		Jun - Dec	
	0-6	SM, SP-SM	A-3, A-2-4	6.0 - 20.0	3.5-6.0			
	6-23	AM, SP-SM	A-2-4, A-3	6.0 - 20.0	3.5-6.0			
	23-47	SM, SP-SM	A-3, A-2-4	6.0 - 20.0	3.5-7.3	0.5 - 1.5	Jun - Nov	
	47-55	SM, SP-SM	A-2-4, A-3	0.6 - 6.0	3.5-7.8			
(20)	55-80	SM, SC	A-2-4, A-6, A-4	0.1 - 0.2	3.5-7.8			
EauGallie	0-5	SP-SM, SM	A-2-4, A-3	6.0 - 20.0	3.5-6.0			
EauGallie Wet	5-17	SP-SM. SM	A-3. A-2-4	6.0 - 20.0	3.5-6.0			
	17-26	SP-SM, SM	A-2-4, A-3	6.0 - 20.0	3.5-7.3			
	26-48	SP-SM, SM	A-2-3, A-3	0.6 - 2.0	3.5-7.8	0.3 - 1.5	Jul - Oct	
	48-72	SP-SM. SM	A-3. A-2-4	0.6 - 2.0	3.5-7.8			
	72-80	SC. CL. SC-SM	A-4, A-6, A-2-4	0.6 - 2.0	3.5-7.8			
	0-19	SP-SM, SM	A-3. A-2-4	6.0 - 20.0	5.6-7.8			
	19-36	SP-SM SP	A-3	6.0 - 20.0	56-78		Jan - Feb	
	36-63	SC-SM SC	A-2-6 A-2-4	01 - 20	56-78	+2.0 - 0.0	Jun - Dec	
(26)	63-80	SP-SM_SM	A-3 A-2-4	60 - 200	56-78			
(20) Floridana	0-10	SP-SM_SP	A-3	60 - 200	45-55			
Immokalee	10-34	SP-SM_SP	A-3	60 - 200	4 5-5 5		Jan - Feb	
Okeelanta	34-43	SP-SM_SM	A-3 A-2-4	0.6 - 20	4 5-5 5	+2.0 - 0.0	Jun - Dec	
	43-80	SP-SM_SP	A-3	60 - 200	4 5-5 5			
	0-20	PT	A-8	6.0 - 20.0	56-84			
	20-54	SP-SM_SP_SM	A-3 A-2-4	60 - 200	56-84	+1.0 - 0.0	Jun - Oct	
	0-5	SP SP-SM	A-3	60 - 200	4.5-6.0			
	5-16	SM_SP-SM	A-2-4 A-3	0.6 - 20	45-60			
	16-52	SP SP-SM	A-3	60 - 200	4.5-6.0	05-15	.lun - Nov	
	52-68	SM SP-SM	Δ_2_4	0.0 - 20.0	4.5-6.0	0.0 1.0		
(35)	68-80	SM SP-SM	A-2-4	01 - 06	45-60			
Ona Fine Sand, non	0-5	SP SP-SM	Δ-3		4 5-6 0			
hydric and hydric	5_16		Δ_2_4 Δ_3	0.0 - 20.0	4560			
	16_52	SP SP_SM	Δ_2	60 - 200	45-60	00-10	Jun - Nov	
	52_69		Δ_2_4		4.5-0.0	0.0 - 1.0		
	68 90	SIVI, SF-SIVI	Δ_2 4	0.1 - 0.2	4.5-0.0			
(45)	00-00	SM SP-SM	Δ_3 Δ_2_1		35.60			
(40) Tavares Fine Sand	6_90			60 200	3560	1.5 - 3.5	Jun - Oct	
	0-00	35-311, 311	A-3, A-2-4	0.0 - 20.0	3.5-0.0			

# **APPENDIX B**

Roadway Soil Survey Boring Location Plan Roadway Soil Profiles Pond Soil Survey Debris Soil Profiles Debris Delineation Plan Muck Delineation Plan

#### MANATEE COUNTY, FLORIDA

DATE OF SURVEY: JULY 2022 TO JUNE 2023 TIERRA, INC. SURVEY MADE BY: KEVIN H. SCOTT, P.E. SUBMITTED BY:

THE MATERIAL FROM STRATUM 1 AND 2 (A-3/A-2-4) APPEARS SATISFACTORY FOR USE IN THE EMBANKMENT WHEN UTILIZED IN ACCORDANCE WITH STANDARD PLANS, INDEX 120-001.

THE MATERIAL FROM STRATUM 3 (A-2-4) APPEARS SATISFACTORY FOR USE IN THE EMBANKMENT WHEN UTILIZED IN ACCORDANCE WITH STANDARD PLANS, INDEX 120-001. HOWEVER, THIS MATERIAL IS LIKELY TO RETAIN EXCESS MOISTURE AND MAY BE DIFFICULT TO DRY AND COMPACT. IT SHOULD BE USED IN THE EMBANKMENT ABOVE THE WATER LEVEL EXISTING AT THE TIME OF CONSTRUCTION.

THE MATERIAL FROM STRATUM 4 (A-2-6/A-4/A-6) IS PLASTIC MATERIAL AND SHALL BE REMOVED IN ACCORDANCE WITH STANDARD PLANS, INDEX 120-002 AND UTILIZED IN

THE MATERIAL FROM STRATUM 5 (A-8) IS MUCK MATERIAL AND SHALL BE REMOVED IN ACCORDANCE WITH STANDARD PLANS, INDEX 120-002 AND UTILIZED IN ACCORDANCE WITH STANDARD PLANS, INDEX 120-001. THE REMOVAL LIMITS ARE SHOWN

THE MATERIAL FROM STRATUM 6 (A-7-6/A-7-5) IS HIGH PLASTIC MATERIAL AND SHALL BE REMOVED IN ACCORDANCE WITH STANDARD PLANS, INDEX 120-002 AND UTILIZED IN ACCORDANCE WITH STANDARD PLANS, INDEX 120-001.

THE MATERIAL FROM STRATUM 7 IS A NATURAL LIMESTONE FORMATION. SPECIAL TOOLS AND EQUIPMENT WILL BE REQUIRED TO EXCAVATE AND/OR DEWATER THIS MATERIAL.

ACCORDANCE WITH STANDARD PLANS, INDEX 120-001.

ON THE ROADWAY CROSS SECTIONS.

#### MANATEE COUNTY PROJECT #6107560 PROJECT NAME: LENA ROAD FROM NORTH OF 44TH AVENUE EAST TO SR 64

#### CROSS SECTION SOIL SURVEY FOR THE DESIGN OF ROADS

SURVEY BEGINS STA. : 218+28.36 SURVEY ENDS STA. : 309+85.00 REFERENCE: CENTERLINE CONSTRUCTION LENA ROAD

	ORG CON	ANIC ITENT	MOI: CON	STURE TENT		SI	EVE ANAL PERCENT	YSIS RESU PASS (%	JLTS )			ATTERBE LIMITS (9	RG %)	-	
STRATUM NO.	NO. OF TESTS	% ORGANIC	NO. OF TESTS	MOISTURE CONTENT	NO. OF TESTS	10 MESH	40 MESH	60 MESH	100 MESH	200 MESH	NO. OF TESTS	LIQUID LIMIT	PLASTIC INDEX	C AASHTO GROUP	DESCRIPTION
1	8	2-4	11	19-27	51	82-100	72-95	54-77	26-41	2-10	2	NP	NP	A-3	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT
2	2	4	4	20-27	11	100	92-95	74-78	40-42	11-14	2	NP	NP	A-2-4	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND
3			14	12-27	20	91	83	69	45	15-35	13	NP-26	NP-10	A-2-4	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYE SAND
4			10	20-46	12	100	97	89	70	21-78	10	NP-39	NP-19	A-2-6/A-6/ A-4	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY TO SILT
5	10	8-28	10	41-133	10					10-32				A-8	DARK GRAY TO BLACK ORGANIC SAND TO ORGANIC SILTY SAND TO PEAT
6			5	46-82	5					44-99	5	47-101	22-46	A-7-6/A-7-5	GRAY TO BROWN SILT TO CLAY
7															CALCAREOUS CLAY TO WEATHERED LIMESTONE
8															LANDFILL DEBRIS

NOTES:

1.

2.

З.

4.

5.

6.

#### EMBANKMENT AND SUBGRADE MATERIAL

STRATA BOUNDARIES ARE APPROXIMATE. MAKE FINAL CHECK AFTER GRADING.

- ▼ WATER TABLE ENCOUNTERED
- $\mathbf{y}^{\dagger}$  GROUNDWATER TABLE AT OR ABOVE GRADE
- $\bigtriangledown$  ESTIMATED SEASONAL HIGH GROUNDWATER TABLE
- $\bigtriangledown$  estimated seasonal high groundwater table at or above grade
- GNE GROUNDWATER NOT ENCOUNTERED
- NP NON-PLASTIC

	ENCOUN NOT USE OFFSITE WILL BE REQUIRE
8.	CEMENTE BORINGS SHALLOW

9. THE "--"

Image: state stat						KHA PROJECT 148400100	<b>N</b>	LICENSED PROFESSIONAL	
Image: scale AS SHOWN     F.E. EICENSE NOMBER 05514     SCALE AS SHOWN       Image: scale AS SHOWN     TIERRA, INC.     SCOTT, P.E.       Image: scale AS SHOWN     TIERRA, INC.     Image: scale AS SHOWN       Image: scale AS SHOWN     TIERRA, INC.     Image: scale AS SHOWN       Image: scale AS SHOWN     TABPLE TERRACE HIGHWAY     Image: scale AS SHOWN       Image: scale AS SHOWN     Image: scale AS SHOWN     Image: scale AS SHOWN       Image: scale AS SHOWN     Image: scale AS SHOWN     Image: scale AS SHOWN       Image: scale AS SHOWN     Image: scale AS SHOWN     Image: scale AS SHOWN       Image: scale AS SHOWN     Image: scale AS SHOWN     Image: scale AS SHOWN       Image: scale AS SHOWN     Image: scale AS SHOWN     Image: scale AS SHOWN       Image: scale AS SHOWN     Image: scale AS SHOWN     Image: scale AS SHOWN       Image: scale AS SHOWN     Image: scale AS SHOWN     Image: scale AS SHOWN       Image: scale AS SHOWN     Image: scale AS SHOWN     Image: scale AS SHOWN       Image: scale AS SHOWN     Image: scale AS SHOWN     Image: scale AS SHOWN       Image: scale AS SHOWN     Image: scale AS SHOWN     Image: scale AS SHOWN       Image: scale AS SHOWN     Image: scale AS SHOWN     Image: scale AS SHOWN       Image: scale AS SHOWN     Image: scale AS SHOWN     Image: scale AS SHOWN       Image: scale AS SHOWN					KEVIN H. SCOTT, P.E.	DATE 11/2022	Manatee	KEVIN H	
Image: No.     Revisions     Date     By     TAMPA, FLORIDA 33637     Distance     Distance     Distance     Distance     Distance					TIERRA, INC.	SCALE AS SHOWN		SCOTT, P.E.	ROAD
NO. REVISIONS DATE BY TAMPA, FLORIDA 33637 CHECKED BY TB MANATEE COUNTY FL DATE:					7351 TEMPLE TERRACE HIGHWAY	DESIGNED BY BJS DRAWN BY BJS	LENA ROAD	65514	
	No.	REVISIONS	DATE	BY	TAMPA, FLORIDA 33637	CHECKED BY TB	MANATEE COUNTY FL	DATE:	

		CORROSION	TEST RE	SULTS	
	NO. OF TESTS	RESISTIVITY C ohm-cm	CHLORIDE ppm	SULFATES ppm	pН
SAND TO	7	2,300-20,000	30-90	<5-270	7.4-8.1
SILTY SAND					
SILTY-CLAYEY					
O SILT					
SILTY SAND					
<ol> <li>THE MATEI MATERIAL ENCOUNTE NOT USED OFFSITE. E WILL BE D REQUIRED.</li> <li>CEMENTED BORINGS. SHALLOW I THIS MATE</li> </ol>	RIAL FROI WAS ENC RED, THE WITHIN XCAVATI IFFICULT DEWATE SAND/H, THIS MAT ERIAL WIL RIAL WIL	M STRATUM 8 I OUNTERED WIT SE MATERIALS THE PROJECT L ONS INTO AND AND SPECIALIZ RING IN THESE ARDPAN WAS EI ERIAL IS ROCK- EXCAVATIONS I L BE DIFFICULT	S LANDFI HIN PARC SHALL BE IMITS AN THROUGH ED EQUII MATERIA NCOUNTE LIKE AND NTO AND NTO AND	LL DEBRIS. REMOVED D DISPOSE 4 THIS MAT PMENT WILL LS WILL BE RED WITHIN 0 IS LOCAT (OR THROU L REQUIRE	THIS AND D OF ERIAL - BE DIFFICULT. N THE ED AT GH NON
SPECIALIZI	ED EQUIP	MENT.	CHNIQUE	S AND	
9. THE "" IN	DICATES	AN UNMEASURE	D PARAM	IETER.	
					SHEET NUMBER
ROAD	WA Y	SOIL SU	RVE	Y	

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#### LEGEND

5/110 / 0 5/1		
BROWN TO L SILTY SAND	.IGHT BROWN TO GRA (A-2-4)	AY TO LIGHT GRAY
BROWN TO L SILTY-CLAYE	.IGHT BROWN TO GRA Y SAND (A-2-4)	AY SILTY SAND TO
GRAY TO BR SILT (A-2-6/)	OWN CLAYEY SAND T A-6/A-4)	O SANDY CLAY TO
DARK GRAY SILTY SAND	TO BLACK ORGANIC S TO PEAT (A-8)	SAND TO ORGANIC
GRAY TO BR	OWN SILT TO CLAY (	A-7-6)
CALCAREOUS	CLAY TO WEATHERE	D LIMESTONE
LANDFILL DE	BRIS	
WATER		
PAVEMENT A	ND BASE MATERIAL	
AASHTO GRC BY VISUAL R ON SELECTE VISUAL REVI	EVIEW AND LABORAT SAMPLES FOR CON EW.	RMINED ORY TESTING FIRMATION OF
NUMBERS TO SPT VALUE I (UNLESS OTI	) THE LEFT OF BORIN FOR 12 INCHES OF PE HERWISE NOTED).	IGS INDICATE NETRATION
NUMBER OF	BLOWS FOR 4 INCHE	S OF PENETRATION
HAND AUGER	RED TO VERIFY UTILI	TY CLEARANCE
SPLIT-SPOON OF ROD AND	I SAMPLER ADVANCEL ) HAMMER	OUNDER WEIGHT
SPLIT-SPOON OF ROD	SAMPLER ADVANCED	D UNDER WEIGHT
ESTIMATED AT OR ABOV	SEASONAL HIGH GROU E GRADE	UNDWATER TABLE
ESTIMATED	SEASONAL HIGH GRO	UNDWATER TABLE
GROUNDWAT FIELD EXPLO	ER LEVEL ENCOUNTE DRATIONS	RED DURING
GROUNDWAT GRADE DURI	ER LEVEL ENCOUNTE NG FIELD EXPLORATI	RED AT OR ABOVE ONS
GROUNDWAT	ER NOT ENCOUNTERE	ED
N CAVE-IN DUE	TO SHALLOW GROUI	NDWATER INTRUSION
ROCK MATER LANDFILL DE	E TO HAND AUGER RI RIAL, CEMENTED SANI BRIS	EFUSAL ON WOOD, DS/HARDPAN AND/OR
T. CENTERLINE	CONSTRUCTION LENA	A ROAD
REPRESENTA GARMIN ETR	TIVE OF TIERRA USIN EX GPS UNIT. THE BO	IG A HAND-HELD ORINGS INDICATED
WITH AN "*" PROJECT SU	WERE LATER SURVEY RVEYOR. STATION AN	LOCATED BY THE
BORINGS WE FROM FIELD	RE DETERMINED BY GPS AND THE SURVE	TIERRA USING THE EYED COORDINATES
IN CONJUNC PROVIDED B	TION WITH PROJECT Y KIMLEY HORN.	DESIGN FILES
2. THE STANDA WITH A "(1)" HAMMER. TH	RD PENETRATION TE. WERE PERFORMED U E REMAINING BORING	ST BORINGS DENOTED TILIZING A SAFETY SS WERE PERFORMED
OTTEIZING A	SAFETY HAMMER	AUTOMATIC HAMMER
JLAR MATERIALS TIVE DENSITY	SPT N-VALUE (BLOWS/FT.)	SPT N-VALUE (BLOWS/FT.)
LOOSE	LESS THAN 4	LESS THAN 3
UM DENSE	10 to 30	8 to 24
DENSE	GREATER THAN 50	GREATER THAN 40
5 AND CLAYS NSISTENCY	SPT N-VALUE (BLOWS/FT.)	SPT N-VALUE (BLOWS/FT.)
SOFT	LESS THAN 2 2 to 4	LESS THAN 1 1 to 3
-	4 to 8	3 to 6
STIFF	6 10 15 15 to 30	12 to 24
	GREAFER THAN 30	GREATER THAN 24

SHEET NUMBER

ROADWAY SOIL PROFILES (1)

	M-2A STA. 225 + G CONST 16' RT. 4/15/202  GN 5 3	30 STA. 225 + r. Q CONS 26' LT. 3 4/15/202 R [GN 1	$ \begin{array}{c} AB-22i \\ AB-22i \\ STA. 225 \\ CON \\$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	AB-226L ST A. 226 + 12 Q CONST. 11' LT. 11/8/2022   ↓ 1	M-4 STA. 226 + 19 Q CONST. 18' RT. 4/15/2023  GNR 1	M-5 STA. 226 + 80 Q CONST. 19' RT. 4/15/2023    	AB-227R STA. 226 + 93 @ CONST. 13' RT. 11/9/2022 3 3 3	B-227L STA. 227 + 07 Q CONST. 25' LT. 11/7/2022 <b>*</b> NI	M-6 STA. 227 + 28 Ç CONST. 21' RT. 4/15/2023 GNR 1 5 1 5	1. 0 2. 3.
5			CAVE-	IN []		. <u> </u>		 CAVE-IN	-   W		5 4. 5. 10 6. 7.
									4 1 14 21	- <u> </u>	15 W P A-3
94									19-  4		25 H 50/4 30 WH
35									50/6		- 35 ⊻ <sup>+</sup> 2 40 ¥
45									11 6		45 GNR CAVE-IN EFUSA 50
											Q CONS NOTES: GRANU RELAT VERY LOOSE MEDIU
			К Р.	EVIN H. SCOTT, P.E. .E. LICENSE NUMBER 6	5514	KHA PROJECT 148400100 DATE 6/2023		Manatee		LICENSED PROFESSIONAL	DENSI VERY SILTS CON VERY SOFT FIRM STIFF VERY HARD
No.	REVISI	:0N5	DATE BY T	IERRA, INC. 351 TEMPLE TERRACE AMPA, FLORIDA 33637	HIGHWAY 6/3i	SCALE AS SHOWN DESIGNED BY BJS DRAWN BY BJS CHECKED BY TB D/2023 2:29:06 PM	MANATEE COUNTY Default	LENA ROAD	)	SCOTT, P.E. FL LICENSE NUMBER 65514 FL DATE: J:\	6511\2022 Files\6511

1.	BROWN TO LI SAND TO SAN	GHT BROW ID WITH SI	N TO GRA	AY TO LIGHT	GRAY
2.	BROWN TO LI SILTY SAND (	GHT BROW A-2-4)	N TO GRA	AY TO LIGHT	GRAY
3.	BROWN TO LI SILTY-CLAYEY	GHT BROW SAND (A-2	N TO GRA	Y SILTY SAN	D TO
4.	GRAY TO BRO SILT (A-2-6/A-	WN CLAYE 6/A-4)	Y SAND T	O SANDY CL	ΑΥ ΤΟ
5.	DARK GRAY T SILTY SAND T	O BLACK C	DRGANIC S	SAND TO ORG	GANIC
6.	GRAY TO BRO	WN SILT T	O CLAY ()	4-7-6)	
7.	CALCAREOUS	CLAY ΤΟ V	VEATHERE	D LIMESTONI	Ē
3.	LANDFILL DEB	RIS			
v	WATER				
Р	PAVEMENT AN	ID BASE M	ATERIAL		
-3	AASHTO GROU BY VISUAL RE ON SELECTED VISUAL REVIE	JP SYMBOL VIEW AND SAMPLES W.	AS DETE LABORAT FOR CON	RMINED ORY TESTING FIRMATION C	î IF
V	NUMBERS TO SPT VALUE FO (UNLESS OTH	THE LEFT DR 12 INCH ERWISE NC	OF BORIN IES OF PE ITED).	IGS INDICATE NETRATION	Ē
0/4	NUMBER OF E	BLOWS FOR	4 INCHES	S OF PENETR	ATION
IA	HAND AUGERE	ED TO VER	IFY UTILI	TY CLEARANC	Έ
/Η	SPLIT-SPOON OF ROD AND	SAMPLER A HAMMER	ADVANCED	UNDER WEI	GHT
/R	SPLIT-SPOON OF ROD	SAMPLER A	ADVANCED	UNDER WEI	GHT
r'	ESTIMATED S AT OR ABOVE	EASONAL F GRADE	IIGH GROU	JNDWATER T	ABLE
$\nabla$	ESTIMATED S	EASONAL F	IIGH GROU	JNDWATER T	ABLE
V	GROUNDWATE FIELD EXPLO	R LEVEL E. RATIONS	NCOUNTEI	RED DURING	
<b>▼</b> <sup>+</sup>	GROUNDWATE GRADE DURIN	R LEVEL E	NCOUNTE XPLORATI	RED AT OR A ONS	BOVE
NR	GROUNDWATE	R LEVEL N	OT RECOP	RDED	
NE E-IN	GROUNDWATE	R NOT ENG	COUNTERE	D הואז פראוחו	DUSION
JSAL	REFUSAL DUE ROCK MATERI	TO HAND AL, CEMEN	AUGER RE	EFUSAL ON W DS/HARDPAN	OOD, AND/OR
ONST.	LANDFILL DEB CENTERLINE (	RIS CONSTRUCI	TION LENA	ROAD	
ES: 1.	THE BORINGS REPRESENTAT	WERE LOC	CATED IN	THE FIELD B	ELD
	WITH AN "*" I	NERE LATE	R SURVEY	LOCATED B	Y THE
	BORINGS WER	E DETERM	INED BY T	IERRA USING	G THE
	IN CONJUNCT	JPS AND I ION WITH	HE SURVE PROJECT	DESIGN FILE	NATES S
2	THE STANDAR	D DENETD	URN. ATION TE	T BODINGS	DENOTED
2.	WITH A "(1)" V HAMMER. THE	VERE PERF REMAININ	ORMED U G BORING	FILIZING A S	AFETY FORMED
	UTILIZING AN	SAFETY	HAMMER	AUTOMATIC	HAMMER
ANULA	R MATERIALS-	SPT N-V	ALUE	SPT N-VAL	UE
RY LO	OSE	LESS THA	AN 4	LESS THAN	3
OSE DIUM	DENSE	4 to 10 10 to 30		3 to 8 8 to 24	
NSE RY DE	NSE	30 to 50	THAN 50	24 to 40	HAN AO
LTS AI	ND CLAYS	SPT N-V	ALUE	SPT N-VAL	UE
CONSI	STENCY FT	(BLOWS	/FT.) N 2	(BLOWS/F	T.) 1
NFT	, ,	2 to 4		1 to 3	•
чм IFF		4 το 8 8 to 15		зто 6 6 to 12	
RY ST	IFF	15 to 30 GREATFR	THAN 30	12 to 24 GREATER TI	HAN 24

SHEET NUMBER

# WAY SOIL PROFILES (2)



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BROWN TO L SAND TO SA	.IGHT BROWN TO GRA ND WITH SILT (A-3)	AY TO LIGHT GRAY
BROWN TO L SILTY SAND	IGHT BROWN TO GRA (A-2-4)	ay to light gray
BROWN TO L SILTY-CLAYE	IGHT BROWN TO GRA ( SAND (A-2-4)	AY SILTY SAND TO
GRAY TO BR SILT (A-2-6/)	OWN CLAYEY SAND T A-6/A-4)	O SANDY CLAY TO
DARK GRAY SILTY SAND	TO BLACK ORGANIC S TO PEAT (A-8)	SAND TO ORGANIC
GRAY TO BR	OWN SILT TO CLAY (	A-7-6)
CALCAREOUS	CLAY TO WEATHERE	D LIMESTONE
LANDFILL DE	BRIS	
WATER		
PAVEMENT A	ND BASE MATERIAL	
AASHTO GRC BY VISUAL R ON SELECTE VISUAL REVI	DUP SYMBOL AS DETE EVIEW AND LABORAT D SAMPLES FOR CON EW.	RMINED ORY TESTING FIRMATION OF
NUMBERS TC SPT VALUE F (UNLESS OTF	THE LEFT OF BORIN OR 12 INCHES OF PE HERWISE NOTED).	IGS INDICATE NETRATION
NUMBER OF	BLOWS FOR 4 INCHE	S OF PENETRATION
HAND AUGEF	RED TO VERIFY UTILI	TY CLEARANCE
SPLIT-SPOON OF ROD AND	SAMPLER ADVANCED HAMMER	D UNDER WEIGHT
SPLIT-SPOON OF ROD	SAMPLER ADVANCED	D UNDER WEIGHT
ESTIMATED AT OR ABOV	SEASONAL HIGH GROU E GRADE	UNDWATER TABLE
ESTIMATED	SEASONAL HIGH GRO	UNDWATER TABLE
GROUNDWAT FIELD EXPLC	ER LEVEL ENCOUNTE PRATIONS	RED DURING
GROUNDWAT GRADE DURI GROUNDWAT	ER LEVEL ENCOUNTE NG FIELD EXPLORATI ER LEVEL NOT RECOM	RED AT OR ABOVE ONS
GROUNDWAT	ER NOT ENCOUNTERE	ED
N CAVE-IN DUE	TO SHALLOW GROUI	NDWATER INTRUSION
ROCK MATER LANDFILL DE	BIAL, CEMENTED SAND BRIS	DS/HARDPAN AND/OR
ST. CENTERLINE	CONSTRUCTION LENA	A ROAD
: 1. THE BORING REPRESENTA	S WERE LOCATED IN TIVE OF TIERRA USIN	I HE FIELD BY A
WITH AN "*"	WERE LATER SURVEY	LOCATED BY THE
BORINGS WE	RE DETERMINED BY	TIERRA USING THE
IN CONJUNC	GPS AND THE SURVE TION WITH PROJECT ( KIMLEY HORN	DESIGN FILES
2 THE STANDA	RD PENETRATION TE	ST BORINGS DENOTED
WITH A "(1)" HAMMER. TH	WERE PERFORMED U	TILIZING A SAFETY
UTILIZING A	N AUTOMATIC HAMME	ER.
ULAR MATERIALS	SPT N-VALUE	SPT N-VALUE
I IVE DENSITY	(BLOWS/FI.)	(BLOWS/FI.)
E LIM DENSE	4 to 10	3 to 8 8 to 24
E DENSE	30 to 50	24 to 40 GREATER THAN 40
5 AND CLAYS	SPT N-VALUE	SPT N-VALUE
NSISTENCY SOFT	(BLOWS/FT.) LESS THAN 2	(BLOWS/FT.) LESS THAN 1
	2 to 4 4 to 8	1 to 3
	8 to 15	6 to 12
SILFF	GREATER THAN 30	GREATER THAN 24

SHEET NUMBER

## ROADWAY SOIL PROFILES (3)



	BROWN TO LI SAND TO SAN	GHT BROWN TO	GRAY TO LIGHT GRAY -3)	
	BROWN TO LI SILTY SAND (	GHT BROWN TO A-2-4)	GRAY TO LIGHT GRAY	
	BROWN TO LI SILTY-CLAYEY	GHT BROWN TO SAND (A-2-4)	GRAY SILTY SAND TO	
	GRAY TO BRC SILT (A-2-6/A	WN CLAYEY SAN -6/A-4)	D TO SANDY CLAY TO	
	DARK GRAY T	O BLACK ORGAN	IC SAND TO ORGANIC	
	GRAY TO BRC	WN SILT TO CLA	AY (A-7-6)	
	CALCAREOUS	CLAY TO WEATH	ERED LIMESTONE	
	LANDFILL DEE	BRIS		
	WATER			
	AASHTO GROU	ND BASE MATERI. TIP SYMBOL AS D	AL ETERMINED	
	BY VISUAL RE ON SELECTED VISUAL REVIE	VIEW AND LABO SAMPLES FOR C W.	RATORY TESTING CONFIRMATION OF	
	NUMBERS TO SPT VALUE FO (UNLESS OTH	THE LEFT OF BC OR 12 INCHES OF ERWISE NOTED).	DRINGS INDICATE PENETRATION	
	NUMBER OF E	BLOWS FOR 4 INC	CHES OF PENETRATION	
	HAND AUGERI	ED TO VERIFY UT	TILITY CLEARANCE	
	OF ROD AND	SAMPLER ADVAN HAMMER	CED UNDER WEIGHT	
	SPLIT-SPOON OF ROD	SAMPLER ADVAN	CED UNDER WEIGHT	
	ESTIMATED S AT OR ABOVE	EASONAL HIGH C GRADE	GROUNDWATER TABLE	
	ESTIMATED S	EASONAL HIGH C	GROUNDWATER TABLE	
	FIELD EXPLO	R LEVEL ENCOUR RATIONS	NIERED DURING	
	GROUNDWATE GRADE DURIN GROUNDWATE	R LEVEL ENCOUR G FIELD EXPLOR R LEVEL NOT RE	NTERED AT OR ABOVE ATIONS CORDED	
	GROUNDWATE	R NOT ENCOUNT	ERED	
AL	REFUSAL DUE	TO SHALLOW GF	R REFUSAL ON WOOD,	v
	ROCK MATERI LANDFILL DEE	IAL, CEMENTED S BRIS	SANDS/HARDPAN AND/OI	R
5T.	CENTERLINE (	CONSTRUCTION L	ENA ROAD	
. 1.	REPRESENTAT	IVE OF TIERRA U X GPS UNIT THI	JSING A HAND-HELD	
	WITH AN "*" PROJECT SUR	WERE LATER SUR VEYOR. STATION	NEY LOCATED BY THE AND OFFSET FOR THE	
	BORINGS WEP FROM FIELD	RE DETERMINED I GPS AND THE SU	BY TIERRA USING THE RVEYED COORDINATES	
	IN CONJUNCT PROVIDED BY	ION WITH PROJE KIMLEY HORN.	CT DESIGN FILES	
2.	THE STANDAR	RD PENETRATION	TEST BORINGS DENOTE	ĒD
	HAMMER. THE UTILIZING AN	REMAINING BOR AUTOMATIC HA	NINGS WERE PERFORMED MMER.	)
		SAFETY HAMM	ER AUTOMATIC HAMME	R
TIV	E DENSITY	(BLOWS/FT.)	(BLOWS/FT.)	
LO E	OSE	LESS THAN 4 4 to 10	LESS THAN 3 3 to 8	
UM E	DENSE	10 to 30 30 to 50	8 to 24 24 to 40	
DE 5 AM	NSE ID CLAYS	GREATER THAN	50 GREATER THAN 40	
NSI	STENCY	(BLOWS/FT.)	(BLOWS/FT.)	
50	гі	LESS IHAN 2 2 to 4	LESS THAN 1 1 to 3	
		4 to 8 8 to 15	3 to 6 6 to 12	
ST.	IFF	15 to 30   GREATER THAN	12 to 24 30  GREATER THAN 24	

SHEET NUMBER

# ROADWAY SOIL PROFILES (4)



#### LEGEND

	BROWN TO LI SAND TO SAN	GHT BROWN TO	GRAY TO LIGHT GRAY -3)	
	BROWN TO LI SILTY SAND (	GHT BROWN TO A-2-4)	GRAY TO LIGHT GRAY	
	BROWN TO LI SILTY-CLAYEY	GHT BROWN TO SAND (A-2-4)	GRAY SILTY SAND TO	
	GRAY TO BRC SILT (A-2-6/A	WN CLAYEY SAN -6/A-4)	D TO SANDY CLAY TO	
	DARK GRAY T	O BLACK ORGAN	IC SAND TO ORGANIC	
	GRAY TO BRC	WN SILT TO CLA	AY (A-7-6)	
	CALCAREOUS	CLAY TO WEATH	IERED LIMESTONE	
	LANDFILL DEE	BRIS		
	WATER			
	AASHTO GROU	ND BASE MATERI. TIP SYMBOL AS D	AL DETERMINED	
	BY VISUAL RE ON SELECTED VISUAL REVIE	VIEW AND LABO SAMPLES FOR C W.	RATORY TESTING CONFIRMATION OF	
	NUMBERS TO SPT VALUE FO (UNLESS OTH	THE LEFT OF BC OR 12 INCHES OF ERWISE NOTED).	ORINGS INDICATE F PENETRATION	
	NUMBER OF E	BLOWS FOR 4 INC	CHES OF PENETRATION	
	HAND AUGERI	ED TO VERIFY UT	TILITY CLEARANCE	
	OF ROD AND	SAMPLER ADVAN HAMMER	ICED UNDER WEIGHT	
	SPLIT-SPOON OF ROD	SAMPLER ADVAN	ICED UNDER WEIGHT	
	ESTIMATED S AT OR ABOVE	EASONAL HIGH C GRADE	GROUNDWATER TABLE	
	ESTIMATED S	EASONAL HIGH C	GROUNDWATER TABLE	
	FIELD EXPLO	R LEVEL ENCOUR RATIONS	NIERED DURING	
	GROUNDWATE GRADE DURIN GROUNDWATE	R LEVEL ENCOUN G FIELD EXPLOR R LEVEL NOT RE	NTERED AT OR ABOVE ATIONS CORDED	
	GROUNDWATE	R NOT ENCOUNT	TERED	
AL	REFUSAL DUE	TO SHALLOW GF	R REFUSAL ON WOOD,	N
	ROCK MATERI LANDFILL DEE	IAL, CEMENTED S BRIS	SANDS/HARDPAN AND/O	R
5T.	CENTERLINE (	CONSTRUCTION L	LENA ROAD	
. 1.	REPRESENTAT	IVE OF TIERRA U X GPS UNIT THI	USING A HAND-HELD	
	WITH AN "*" PROJECT SUR	WERE LATER SUR VEYOR. STATION	RVEY LOCATED BY THE AND OFFSET FOR THE	
	BORINGS WEP FROM FIELD	RE DETERMINED I GPS AND THE SU	BY TIERRA USING THE IRVEYED COORDINATES	
	IN CONJUNCT PROVIDED BY	ION WITH PROJE KIMLEY HORN.	ECT DESIGN FILES	
2.	THE STANDAR	RD PENETRATION	TEST BORINGS DENOT	ED
	HAMMER. THE UTILIZING AN	REMAINING BOR AUTOMATIC HA	RINGS WERE PERFORMEL MMER.	D
		SAFETY HAMM	ER AUTOMATIC HAMME	ER
TIV	E DENSITY	(BLOWS/FT.)	(BLOWS/FT.)	
LO E	OSE	LESS THAN 4 4 to 10	LESS THAN 3 3 to 8	
UM E	DENSE	10 to 30 30 to 50	8 to 24 24 to 40	
DE 5 AM	NSE ID CLAYS	GREATER THAN	I 50 GREATER THAN 40	
NSI	STENCY	(BLOWS/FT.)	(BLOWS/FT.)	
50	r i	2 to 4	1 to 3	
		4 to 8 8 to 15	3 to 6 6 to 12	
ST.	IFF	15 to 30   GREATER THAN	12 to 24 30 GREATER THAN 24	!

SHEET NUMBER

## ROADWAY SOIL PROFILES (5)



#### *LEGEND*

	BROWN TO LI SAND TO SAN	GHT BROWN TO	GRAY TO LIGHT GRAY -3)	
	BROWN TO LI SILTY SAND (	GHT BROWN TO A-2-4)	GRAY TO LIGHT GRAY	
	BROWN TO LI SILTY-CLAYEY	GHT BROWN TO SAND (A-2-4)	GRAY SILTY SAND TO	
	GRAY TO BRC SILT (A-2-6/A	WN CLAYEY SAN -6/A-4)	D TO SANDY CLAY TO	
	DARK GRAY T	O BLACK ORGAN	IC SAND TO ORGANIC	
	GRAY TO BRC	WN SILT TO CLA	AY (A-7-6)	
	CALCAREOUS	CLAY TO WEATH	IERED LIMESTONE	
	LANDFILL DEE	BRIS		
	WATER			
	AASHTO GROU	ND BASE MATERI. TIP SYMBOL AS D	AL DETERMINED	
	BY VISUAL RE ON SELECTED VISUAL REVIE	VIEW AND LABO SAMPLES FOR C W.	RATORY TESTING CONFIRMATION OF	
	NUMBERS TO SPT VALUE FO (UNLESS OTH	THE LEFT OF BC OR 12 INCHES OF ERWISE NOTED).	ORINGS INDICATE F PENETRATION	
	NUMBER OF E	BLOWS FOR 4 INC	CHES OF PENETRATION	
	HAND AUGERI	ED TO VERIFY UT	TILITY CLEARANCE	
	OF ROD AND	SAMPLER ADVAN HAMMER	ICED UNDER WEIGHT	
	SPLIT-SPOON OF ROD	SAMPLER ADVAN	ICED UNDER WEIGHT	
	ESTIMATED S AT OR ABOVE	EASONAL HIGH C GRADE	GROUNDWATER TABLE	
	ESTIMATED S	EASONAL HIGH C	GROUNDWATER TABLE	
	FIELD EXPLO	R LEVEL ENCOUR RATIONS	NIERED DURING	
	GROUNDWATE GRADE DURIN GROUNDWATE	R LEVEL ENCOUN G FIELD EXPLOR R LEVEL NOT RE	NTERED AT OR ABOVE ATIONS CORDED	
	GROUNDWATE	R NOT ENCOUNT	TERED	
AL	REFUSAL DUE	TO SHALLOW GF	R REFUSAL ON WOOD,	N
	ROCK MATERI LANDFILL DEE	IAL, CEMENTED S BRIS	SANDS/HARDPAN AND/O	R
5T.	CENTERLINE (	CONSTRUCTION L	LENA ROAD	
. 1.	REPRESENTAT	IVE OF TIERRA U X GPS UNIT THI	USING A HAND-HELD	
	WITH AN "*" PROJECT SUR	WERE LATER SUR VEYOR. STATION	RVEY LOCATED BY THE AND OFFSET FOR THE	
	BORINGS WEP FROM FIELD	RE DETERMINED I GPS AND THE SU	BY TIERRA USING THE IRVEYED COORDINATES	
	IN CONJUNCT PROVIDED BY	ION WITH PROJE KIMLEY HORN.	ECT DESIGN FILES	
2.	THE STANDAR	RD PENETRATION	TEST BORINGS DENOT	ED
	HAMMER. THE UTILIZING AN	REMAINING BOR AUTOMATIC HA	RINGS WERE PERFORMEL MMER.	D
		SAFETY HAMM	ER AUTOMATIC HAMME	ER
TIV	E DENSITY	(BLOWS/FT.)	(BLOWS/FT.)	
LO E	OSE	LESS THAN 4 4 to 10	LESS THAN 3 3 to 8	
UM E	DENSE	10 to 30 30 to 50	8 to 24 24 to 40	
DE 5 AM	NSE ID CLAYS	GREATER THAN	I 50 GREATER THAN 40	
NSI	STENCY	(BLOWS/FT.)	(BLOWS/FT.)	
50	r i	2 to 4	1 to 3	
		4 to 8 8 to 15	3 to 6 6 to 12	
ST.	IFF	15 to 30   GREATER THAN	12 to 24 30 GREATER THAN 24	!

SHEET NUMBER

## ROADWAY SOIL PROFILES (6)



#### LEGEND

	BROWN TO LI SAND TO SAN	GHT BROW	'N TO GRA ILT (A-3)	AY TO LIGHT	GRAY
l	BROWN TO LI SILTY SAND (	GHT BROW A-2-4)	N TO GRA	AY TO LIGHT	GRAY
	BROWN TO LI SILTY-CLAYEY	GHT BROW SAND (A-2	'N TO GRA 2-4)	AY SILTY SA	ND TO
(	GRAY TO BRC SILT (A-2-6/A-	WN CLAYE -6/A-4)	Y SAND T	O SANDY C	LAY TO
ļ	DARK GRAY T	O BLACK	ORGANIC	SAND TO OF	RGANIC
(	GRAY TO BRC	WN SILT 1	-0 CLAY (.	A-7-6)	
(	CALCAREOUS	CLAY TO W	VEATHERE	D LIMESTON	νE
l	LANDFILL DEE	BRIS			
l	NATER				
1	PAVEMENT AN AASHTO GROU	ND BASE M IP SYMBOL	ATERIAL	RMINED	
	BY VISUAL RE ON SELECTED VISUAL REVIE	VIEW AND SAMPLES W.	LABORAT FOR CON	ORY TESTIN FIRMATION	'G OF
	NUMBERS TO SPT VALUE FO (UNLESS OTH)	THE LEFT OR 12 INCH ERWISE NO	OF BORIN IES OF PE DTED).	IGS INDICAT INETRATION	Ē
I	NUMBER OF E	BLOWS FOR	4 INCHE	S OF PENET	RATION
1	HAND AUGERI	ED TO VER	IFY UTILI	TY CLEARAN	CE
i	OF ROD AND	SAMPLER . HAMMER	ADV ANCEL	O UNDER WE	IGHI
	SPLIT-SPOON OF ROD	SAMPLER .	ADVANCEL	OUNDER WE	IGHT
ļ	ESTIMATED S AT OR ABOVE	EASONAL I GRADE	HIGH GRO	UNDWATER	TABLE
l	ESTIMATED S	EASONAL I	HIGH GRO	UNDWATER	TABLE
i I	GROUNDWATE FIELD EXPLOI	R LEVEL E RATIONS	NCOUNTE	RED DURING	i
0	GROUNDWATE GRADE DURIN	R LEVEL E G FIELD E	NCOUNTE XPLORATI	RED AT OR ONS	ABOVE
· (	GROUNDWATE GROUNDWATE	R LEVEL N R NOT EN	COUNTERE	RDED ED	
N (	CAVE-IN DUE	TO SHALL	OW GROUI	NDWATER IN	ITRUSION
4L     	REFUSAL DUE ROCK MATERI LANDFILL DEE	TO HAND AL, CEMEN BRIS	AUGER RI ITED SANI	EFUSAL ON DS/HARDPAN	WOOD, I AND/OR
5T. (	CENTERLINE ( THE BORINGS	CONSTRUC	TION LENA	A ROAD	BY A
	REPRESENTAT GARMIN ETRE	IVE OF TI X GPS UN	ERRA USIN T. THE BO	IG A HAND-I DRINGS IND	HELD ICATED
V F	NITH AN "*" PROJECT SUR	NERE LATE VEYOR. ST	R SURVEY	' LOCATED I D OFFSET F	BY THE OR THE
E	BORINGS WER FROM FIELD (	RE DETERM GPS AND T	INED BY HE SURVE	TIERRA USIN YED COORD	IG THE VINATES
ļ	PROVIDED BY	ION WITH KIMLEY H	PROJECT ORN.	DESIGN FIL	ES
2. T V H	THE STANDAR WITH A "(1)" V HAMMER. THE	RD PENETR VERE PERF REMAININ AUTOMAT	ATION TE ORMED U G BORING	ST BORINGS TILIZING A S WERE PEI	DENOTED SAFETY RFORMED
		SAFETY	HAMMER	AUTOMATIC	C HAMMER
JLAR TIVE	MATERIALS- DENSITY	SPT N-V (BLOWS	/ALUE /FT.)	SPT N-VA (BLOWS/H	LUE T.)
LOC E	)SE	LESS TH, 4 to 10	AN 4	LESS THAN 3 to 8	13
ŪM L F	DENSE	10 to 30 30 to 50		8 to 24 24 to 40	
	ISE	GREATER	THAN 50	GREATER T	THAN 40
NSIS	TENCY	(BLOWS	S/FT.)	(BLOWS/	FT.)
SOF	T	LESS THA 2 to 4	AN 2	LESS THAN   1 to 3	11
-		4 to 8 8 to 15		3 to 6 6 to 12	
STI	FF	15 to 30 GREATER	THAN 30	12 to 24 GREATER	THAN 24
		2			

SHEET NUMBER

ROADWAY SOIL PROFILES (7)



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#### *LEGEND*

	BROWN TO LI SAND TO SAN	GHT BROW	'N TO GR ILT (A-3)	AY TO LIGHT	GRAY
	BROWN TO LI SILTY SAND (	GHT BROW A-2-4)	'N TO GR	AY TO LIGHT	GRAY
	BROWN TO LI SILTY-CLAYEY	GHT BROW SAND (A-2	'N TO GR/ ?-4)	AY SILTY SAN	D ТО
	GRAY TO BRC SILT (A-2-6/A	WN CLAYE -6/A-4)	Y SAND T	O SANDY CLA	ΑΥ ΤΟ
	DARK GRAY T SILTY SAND T	O BLACK (	DRGANIC	SAND TO ORG	SANIC
	GRAY TO BRC	WN SILT T	O CLAY (	A-7-6)	
	CALCAREOUS	CLAY TO V	VEATHERE	D LIMESTONE	Ŧ
	LANDFILL DEE	BRIS			
	WATER				
	PAVEMENT A	VD BASE M	ATERIAL		
	AASHTO GROU BY VISUAL RE ON SELECTED VISUAL REVIE	JP SYMBOL EVIEW AND SAMPLES EW.	LABORAT FOR CON	RMINED ORY TESTING FIRMATION O	F
	NUMBERS TO SPT VALUE FO (UNLESS OTH	THE LEFT OR 12 INCH ERWISE NO	OF BORII IES OF PE DTED).	NGS INDICATE ENETRATION	-
	NUMBER OF E	BLOWS FOR	4 INCHE	S OF PENETR	ATION
	HAND AUGERI	ED TO VER	IFY UTILI	TY CLEARANC	E
	SPLIT-SPOON OF ROD AND	SAMPLER A HAMMER	ADVANCEL	D UNDER WEI	GHT
	SPLIT-SPOON OF ROD	SAMPLER A	ADVANCEL	D UNDER WEI	GHT
	ESTIMATED S AT OR ABOVE	EASONAL F GRADE	HIGH GRO	UNDWATER T	ABLE
	ESTIMATED S	EASONAL H	HIGH GRO	UNDWATER TA	ABLE
	GROUNDWATE	ER LEVEL E RATIONS	NCOUNTE	RED DURING	
	GROUNDWATE GRADE DURIN	R LEVEL E	NCOUNTE XPLORATI	RED AT OR AN ONS	BOVE
	GROUNDWATE	R NOT EN	COUNTER	ED	
N	CAVE-IN DUE	TO SHALL	OW GROU	NDWATER INT	RUSION
42	ROCK MATERI LANDFILL DEE	IO HAND IAL, CEMEN BRIS	AUGER R ITED SANI	EFUSAL ON W DS/HARDPAN	OOD, AND/OR
5T.	CENTERLINE (		TION LEN	A ROAD	× •
; 1.	REPRESENTAT	IVE OF TIE X GPS UNI	ERRA USIN T. THE B	I HE FIELD B NG A HAND-HE ORINGS INDIC	Y A ELD CATED
	WITH AN "*" PROJECT SUR	WERE LATE VEYOR. ST	R SURVEY	/ LOCATED BY D OFFSET FO	' THE R THE
	BORINGS WEF	RE DETERM GPS AND T	INED BY HE SURVE	TIERRA USING YED COORDII	THE NATES
	IN CONJUNCT PROVIDED BY	ION WITH KIMLEY H	PROJECT ORN.	DESIGN FILES	5
2.	THE STANDAR WITH A "(1)" V HAMMER. THE	RD PENETR NERE PERF REMAININ	ATION TE ORMED U G BORING	ST BORINGS I TILIZING A S. SS WERE PERF	DENOTED AFETY FORMED
	OTIEIZING AN	SAFETY	HAMMER	AUTOMATIC	HAMMER
ULAI TIVI	R MATERIALS- E DENSITY	SPT N-V (BLOWS)	ALUE /FT.)	SPT N-VAL (BLOWS/FT	UE .)
LO	OSE	LESS THA	AN 4	LESS THAN	3
ŪM F	DENSE	10 to 30		8 to 24 24 to 40	
	NSE	GREATER	THAN 50	GREATER TH	AN 40
NSI:	ND CLAYS STENCY	SPT N-V (BLOWS	ALUE /FT.)	BLOWS/FT	БЕ Г.)
50	FT	LESS THA 2 to 4	N 2	LESS THAN 1 to 3	1
-		4 to 8 8 to 15		3 to 6	
ST	IFF	15 to 30	TUAN 20	12 to 24	IAN 24
		GREATER	INAN 30	GREATER IF	TAN 24

SHEET NUMBER

# ROADWAY SOIL PROFILES (8)



	BROWN TO LI SAND TO SAN	GHT BROWN TO	GRAY TO LIGHT GRAY -3)	
	BROWN TO LI SILTY SAND (	GHT BROWN TO A-2-4)	GRAY TO LIGHT GRAY	
	BROWN TO LI SILTY-CLAYEY	GHT BROWN TO SAND (A-2-4)	GRAY SILTY SAND TO	
	GRAY TO BRC SILT (A-2-6/A	WN CLAYEY SAN -6/A-4)	D TO SANDY CLAY TO	
	DARK GRAY T	O BLACK ORGAN	IC SAND TO ORGANIC	
	GRAY TO BRC	WN SILT TO CLA	AY (A-7-6)	
	CALCAREOUS	CLAY TO WEATH	IERED LIMESTONE	
	LANDFILL DEE	BRIS		
	WATER			
	AASHTO GROU	ND BASE MATERI. TIP SYMBOL AS D	AL DETERMINED	
	BY VISUAL RE ON SELECTED VISUAL REVIE	VIEW AND LABO SAMPLES FOR C W.	RATORY TESTING CONFIRMATION OF	
	NUMBERS TO SPT VALUE FO (UNLESS OTH	THE LEFT OF BC OR 12 INCHES OF ERWISE NOTED).	ORINGS INDICATE F PENETRATION	
	NUMBER OF E	BLOWS FOR 4 INC	CHES OF PENETRATION	
	HAND AUGERI	ED TO VERIFY UT	TILITY CLEARANCE	
	OF ROD AND	SAMPLER ADVAN HAMMER	ICED UNDER WEIGHT	
	SPLIT-SPOON OF ROD	SAMPLER ADVAN	ICED UNDER WEIGHT	
	ESTIMATED S AT OR ABOVE	EASONAL HIGH C GRADE	GROUNDWATER TABLE	
	ESTIMATED S	EASONAL HIGH C	GROUNDWATER TABLE	
	FIELD EXPLO	R LEVEL ENCOUR RATIONS	NIERED DURING	
	GROUNDWATE GRADE DURIN GROUNDWATE	R LEVEL ENCOUN G FIELD EXPLOR R LEVEL NOT RE	NTERED AT OR ABOVE ATIONS CORDED	
	GROUNDWATE	R NOT ENCOUNT	TERED	
AL	REFUSAL DUE	TO SHALLOW GF	R REFUSAL ON WOOD,	N
	ROCK MATERI LANDFILL DEE	IAL, CEMENTED S BRIS	SANDS/HARDPAN AND/O	R
5T.	CENTERLINE (	CONSTRUCTION L	LENA ROAD	
. 1.	REPRESENTAT	IVE OF TIERRA U X GPS UNIT THI	USING A HAND-HELD	
	WITH AN "*" PROJECT SUR	WERE LATER SUR VEYOR. STATION	RVEY LOCATED BY THE AND OFFSET FOR THE	
	BORINGS WEP FROM FIELD	RE DETERMINED I GPS AND THE SU	BY TIERRA USING THE IRVEYED COORDINATES	
	IN CONJUNCT PROVIDED BY	ION WITH PROJE KIMLEY HORN.	ECT DESIGN FILES	
2.	THE STANDAR	RD PENETRATION	TEST BORINGS DENOT	ED
	HAMMER. THE UTILIZING AN	REMAINING BOR AUTOMATIC HA	RINGS WERE PERFORMEL MMER.	D
		SAFETY HAMM	ER AUTOMATIC HAMME	ER
TIV	E DENSITY	(BLOWS/FT.)	(BLOWS/FT.)	
LO E	OSE	LESS THAN 4 4 to 10	LESS THAN 3 3 to 8	
UM E	DENSE	10 to 30 30 to 50	8 to 24 24 to 40	
DE 5 AM	NSE ID CLAYS	GREATER THAN	I 50 GREATER THAN 40	
NSI	STENCY	(BLOWS/FT.)	(BLOWS/FT.)	
50	r I	2 to 4	1 to 3	
		4 to 8 8 to 15	3 to 6 6 to 12	
ST.	IFF	15 to 30   GREATER THAN	12 to 24 30 GREATER THAN 24	!

ROADWAY SOIL PROFILES (9)

SHEET NUMBER


ult		
unc		

BROWN TO LI SAND TO SAN	GHT BROWN TO GRA ID WITH SILT (A-3)	AY TO LIGHT GRAY
BROWN TO LI SILTY SAND (	GHT BROWN TO GRA A-2-4)	ay to light gray
BROWN TO LI SILTY-CLAYEY	GHT BROWN TO GRA SAND (A-2-4)	AY SILTY SAND TO
GRAY TO BRC SILT (A-2-6/A	DWN CLAYEY SAND T -6/A-4)	O SANDY CLAY TO
DARK GRAY T SILTY SAND	O BLACK ORGANIC S	SAND TO ORGANIC
GRAY TO BRC	OWN SILT TO CLAY (	A-7-6)
CALCAREOUS	CLAY TO WEATHERE	D LIMESTONE
LANDFILL DEE	BRIS	
VVAIER PAVEMENT AI	ND BASE MATERIAL	
AASHTO GROU BY VISUAL RE ON SELECTED VISUAL REVIE	UP SYMBOL AS DETE EVIEW AND LABORAT O SAMPLES FOR CON W.	RMINED ORY TESTING FIRMATION OF
NUMBERS TO SPT VALUE F (UNLESS OTH	THE LEFT OF BORIN OR 12 INCHES OF PE ERWISE NOTED).	IGS INDICATE NETRATION
NUMBER OF E	BLOWS FOR 4 INCHES	S OF PENETRATION
HAND AUGER	ED TO VERIFY UTILI	TY CLEARANCE
SPLIT-SPOON OF ROD AND	SAMPLER ADVANCED HAMMER	OUNDER WEIGHT
SPLIT-SPOON OF ROD	SAMPLER ADVANCED	DUNDER WEIGHT
ESTIMATED S AT OR ABOVE	EASONAL HIGH GROU GRADE	UNDWATER TABLE
ESTIMATED S	EASONAL HIGH GROU	UNDWATER TABLE
GROUNDWATE FIELD EXPLO	R LEVEL ENCOUNTER RATIONS	RED DURING
GROUNDWATE GRADE DURIN	R LEVEL ENCOUNTER G FIELD EXPLORATION	RED AT OR ABOVE ONS
GROUNDWATE GROUNDWATE	R LEVEL NOT RECOP R NOT ENCOUNTERE	RDED ED
N CAVE-IN DUE	TO SHALLOW GROUI	NDWATER INTRUSION
AL REFUSAL DUE ROCK MATER LANDFILL DEE	TO HAND AUGER RE IAL, CEMENTED SAND BRIS	EFUSAL ON WOOD, DS/HARDPAN AND/OR
ST. CENTERLINE	CONSTRUCTION LENA	A ROAD
REPRESENTAT	IVE OF TIERRA USIN X GPS UNIT. THE BO	I HE FIELD BY A IG A HAND-HELD DRINGS INDICATED
WITH AN "*" PROJECT SUR	WERE LATER SURVEY VEYOR. STATION ANI	LOCATED BY THE D OFFSET FOR THE
BORINGS WEF	RE DETERMINED BY T GPS AND THE SURVE	TIERRA USING THE YED COORDINATES
PROVIDED BY	KIMLEY HORN.	DESIGN FILES
2. THE STANDAF WITH A "(1)" \ HAMMER. THE UTUIZING AN	RD PENETRATION TES NERE PERFORMED U REMAINING BORING AUTOMATIC HAMME	ST BORINGS DENOTED TILIZING A SAFETY S WERE PERFORMED
0112121110 ///	SAFETY HAMMER	AUTOMATIC HAMMER
ULAR MATERIALS- TIVE DENSITY	SPT N-VALUE (BLOWS/FT.)	SPT N-VALUE (BLOWS/FT.)
LOOSE E	LESS THAN 4 4 to 10	LESS THAN 3 3 to 8
UM DENSE E	10 to 30 30 to 50	8 to 24 24 to 40
DENSE 5 AND CLAYS	GREATER THAN 50 SPT N-VALUE	GREATER THAN 40 SPT N-VALUE
NSISTENCY	(BLOWS/FT.)	(BLOWS/FT.)
5011	2 to 4	1 to 3
- CTIEE	8 to 15	6 to 12
51155	GREATER THAN 30	GREATER THAN 24

SHEET NUMBER

## ROADWAY SOIL PROFILES (10)



	LEGEND
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT (A-3)
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND (A-2-4)
	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYEY SAND (A-2-4)
	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY (A-2-6/A-6/A-4)
	DARK GRAY TO BLACK ORGANIC SAND TO ORGANIC SILTY SAND TO PEAT (A-8)
	GRAY TO BROWN SILT TO CLAY (A-7-6)
	CALCAREOUS CLAY TO WEATHERED LIMESTONE
	AASHTO GROUP SYMBOL AS DETERMINED BY VISUAL REVIEW AND LABORATORY TESTING ON SELECTED SAMPLES FOR CONFIRMATION OF VISUAL REVIEW.
	NUMBERS TO THE LEFT OF BORINGS INDICATE SPT VALUE FOR 12 INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).
	NUMBER OF BLOWS FOR 4 INCHES OF PENETRATION
	HAND AUGERED TO VERIFY UTILITY CLEARANCE
	PERCENT PASSING #200 SIEVE NATURAL MOISTURE CONTENT (%) LIQUID LIMIT (%) PLASTICITY INDEX (%) ORGANIC CONTENT (%) NON-PLASTIC
8	NORTH AMERICAN VERTICAL DATUM OF 1988
	APPROXIMATE SPT BORING LOCATION
	APPROXIMATE AUGER BORING LOCATION
	APPROXIMATE DRIT LOCATION
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE AT OR ABOVE GRADE
	GROUNDWATER LEVEL ENCOUNTERED DURING FIELD EXPLORATIONS
	GROUNDWATER LEVEL ENCOUNTERED AT OR ABOVE GRADE DURING FIELD EXPLORATIONS
I	BORING TERMINATED DUE TO CAVE-IN FROM GROUNDWATER INTRUSION

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	SAFETY HAMMER	AUTOMATIC HAMMER	TH
NULAR MATERIALS- ATIVE DENSITY	SPT N-VALUE (BLOWS/FT.)	SPT N-VALUE (BLOWS/FT.)	IS .
Y LOOSE SE IUM DENSE SE Y DENSE	LESS THAN 4 4 to 10 10 to 30 30 to 50 GREATER THAN 50	LESS THAN 3 3 to 8 8 to 24 24 to 40 GREATER THAN 40	HIS SHEET
S AND CLAYS ONSISTENCY	SPT N-VALUE (BLOWS/FT.)	SPT N-VALUE (BLOWS/FT.)	OF T
Y SOFT T 1 F F Y STIFF D	LESS THAN 2 2 to 4 4 to 8 8 to 15 15 to 30 GREATER THAN 30	LESS THAN 1 1 to 3 3 to 6 6 to 12 12 to 24 GREATER THAN 24	CIAL RECORD
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#### POND SOIL SURVEY (1)



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	LEGEND
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT (A-3)
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND (A-2-4)
	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYEY SAND (A-2-4)
	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY (A-2-6/A-6/A-4)
	DARK GRAY TO BLACK ORGANIC SAND TO ORGANIC SILTY SAND TO PEAT (A-8)
	GRAY TO BROWN SILT TO CLAY (A-7-6)
	CALCAREOUS CLAY TO WEATHERED LIMESTONE
	AASHTO GROUP SYMBOL AS DETERMINED BY VISUAL REVIEW AND LABORATORY TESTING ON SELECTED SAMPLES FOR CONFIRMATION OF VISUAL REVIEW.
	NUMBERS TO THE LEFT OF BORINGS INDICATE SPT VALUE FOR 12 INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).
	NUMBER OF BLOWS FOR 4 INCHES OF PENETRATION
	HAND AUGERED TO VERIFY UTILITY CLEARANCE
	PERCENT PASSING #200 SIEVE NATURAL MOISTURE CONTENT (%) LIQUID LIMIT (%) PLASTICITY INDEX (%) ORGANIC CONTENT (%) NON-PLASTIC
88	NORTH AMERICAN VERTICAL DATUM OF 1988
	APPROXIMATE SPT BORING LOCATION
	APPROXIMATE AUGER BORING LOCATION
	APPROXIMATE DRIT LOCATION
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE AT OR ABOVE GRADE
	GROUNDWATER LEVEL ENCOUNTERED DURING FIELD EXPLORATIONS
	GROUNDWATER LEVEL ENCOUNTERED AT OR ABOVE GRADE DURING FIELD EXPLORATIONS
J	BORING TERMINATED DUE TO CAVE-IN FROM GROUNDWATER INTRUSION
L	BORING TERMINATED DUE TO HAND AUGER REFUSAL ON CEMENTED SANDS/HARDPAN AND/OR ROCK MATERIAL

Q CONST. CENTERLINE CONSTRUCTION LENA ROAD

	SAFETY HAMMER	AUTOMATIC HAMMER
NULAR MATERIALS-	SPT N-VALUE	SPT N-VALUE
ATIVE DENSITY	(BLOWS/FT.)	(BLOWS/FT.)
Y LOOSE	LESS THAN 4	LESS THAN 3
SE	4 to 10	3 to 8
DIUM DENSE	10 to 30	8 to 24
SE	30 to 50	24 to 40
Y DENSE	GREATER THAN 50	GREATER THAN 40
S AND CLAYS	SPT N-VALUE	SPT N-VALUE
ONSISTENCY	(BLOWS/FT.)	(BLOWS/FT.)
Y SOFT T 4 F F Y STIFF D	LESS THAN 2 2 to 4 4 to 8 8 to 15 15 to 30 GREATER THAN 30	LESS THAN 1 1 to 3 3 to 6 6 to 12 12 to 24 GREATER THAN 24

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## POND SOIL SURVEY (2)



LEGEND	

	LEGEND
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT (A-3)
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND (A-2-4)
	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYEY SAND (A-2-4)
	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY (A-2-6/A-6/A-4)
	DARK GRAY TO BLACK ORGANIC SAND TO ORGANIC SILTY SAND TO PEAT (A-8)
	GRAY TO BROWN SILT TO CLAY (A-7-6)
	CALCAREOUS CLAY TO WEATHERED LIMESTONE
	AASHTO GROUP SYMBOL AS DETERMINED BY VISUAL REVIEW AND LABORATORY TESTING ON SELECTED SAMPLES FOR CONFIRMATION OF VISUAL REVIEW.
	NUMBERS TO THE LEFT OF BORINGS INDICATE SPT VALUE FOR 12 INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).
	NUMBER OF BLOWS FOR 4 INCHES OF PENETRATION
	HAND AUGERED TO VERIFY UTILITY CLEARANCE
	PERCENT PASSING #200 SIEVE NATURAL MOISTURE CONTENT (%) LIQUID LIMIT (%) PLASTICITY INDEX (%) ORGANIC CONTENT (%) NON-PLASTIC
8	NORTH AMERICAN VERTICAL DATUM OF 1988
	APPROXIMATE SPT BORING LOCATION
	APPROXIMATE AUGER BORING LOCATION
	APPROXIMATE DRIT LOCATION
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE AT OR ABOVE GRADE
	GROUNDWATER LEVEL ENCOUNTERED DURING FIELD EXPLORATIONS
	GROUNDWATER LEVEL ENCOUNTERED AT OR ABOVE GRADE DURING FIELD EXPLORATIONS
1	BORING TERMINATED DUE TO CAVE-IN FROM GROUNDWATER INTRUSION

	SAFETY HAMMER	AUTOMATIC HAMMER
NULAR MATERIALS-	SPT N-VALUE	SPT N-VALUE
ATIVE DENSITY	(BLOWS/FT.)	(BLOWS/FT.)
Y LOOSE	LESS THAN 4	LESS THAN 3
SE	4 to 10	3 to 8
IUM DENSE	10 to 30	8 to 24
SE	30 to 50	24 to 40
Y DENSE	GREATER THAN 50	GREATER THAN 40
S AND CLAYS	SPT N-VALUE	SPT N-VALUE
ONSISTENCY	(BLOWS/FT.)	(BLOWS/FT.)
Y SOFT	LESS THAN 2	LESS THAN 1
T	2 to 4	1 to 3
1	4 to 8	3 to 6
F	8 to 15	6 to 12
Y STIFF	15 to 30	12 to 24
D	GREATER THAN 30	GREATER THAN 24

POND SOIL SURVEY (3)

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LEGEND	

	LEGEND
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT (A-3)
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND (A-2-4)
	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYEY SAND (A-2-4)
	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY (A-2-6/A-6/A-4)
	DARK GRAY TO BLACK ORGANIC SAND TO ORGANIC SILTY SAND TO PEAT (A-8)
	GRAY TO BROWN SILT TO CLAY (A-7-6)
	CALCAREOUS CLAY TO WEATHERED LIMESTONE
	AASHTO GROUP SYMBOL AS DETERMINED BY VISUAL REVIEW AND LABORATORY TESTING ON SELECTED SAMPLES FOR CONFIRMATION OF VISUAL REVIEW.
	NUMBERS TO THE LEFT OF BORINGS INDICATE SPT VALUE FOR 12 INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).
	NUMBER OF BLOWS FOR 4 INCHES OF PENETRATION
	HAND AUGERED TO VERIFY UTILITY CLEARANCE
	PERCENT PASSING #200 SIEVE NATURAL MOISTURE CONTENT (%) LIQUID LIMIT (%) PLASTICITY INDEX (%) ORGANIC CONTENT (%) NON-PLASTIC
8	NORTH AMERICAN VERTICAL DATUM OF 1988
	APPROXIMATE SPT BORING LOCATION
	APPROXIMATE AUGER BORING LOCATION
	APPROXIMATE DRIT LOCATION
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE AT OR ABOVE GRADE
	GROUNDWATER LEVEL ENCOUNTERED DURING FIELD EXPLORATIONS
	GROUNDWATER LEVEL ENCOUNTERED AT OR ABOVE GRADE DURING FIELD EXPLORATIONS
1	BORING TERMINATED DUE TO CAVE-IN FROM GROUNDWATER INTRUSION

	SAFETY HAMMER	AUTOMATIC HAMMER
NULAR MATERIALS- ATIVE DENSITY	SPT N-VALUE (BLOWS/FT.)	SPT N-VALUE (BLOWS/FT.)
Y LOOSE	LESS THAN 4	LESS THAN 3
SE	4 to 10	3 to 8
IUM DENSE	10 to 30	8 to 24
SE	30 to 50	24 to 40
Y DENSE	GREATER THAN 50	GREATER THAN 40
S AND CLAYS	SPT N-VALUE	SPT N-VALUE
ONSISTENCY	(BLOWS/FT.)	(BLOWS/FT.)
Y SOFT	LESS THAN 2	LESS THAN 1
Т	2 to 4	1 to 3
1	4 to 8	3 to 6
F	8 to 15	6 to 12
Y STIFF	15 to 30	12 to 24
D	GREATER THAN 30	GREATER THAN 24

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#### POND SOIL SURVEY (4)



LEGEND	

	LEGEND
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT (A-3)
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND (A-2-4)
	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYEY SAND (A-2-4)
	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY (A-2-6/A-6/A-4)
	DARK GRAY TO BLACK ORGANIC SAND TO ORGANIC SILTY SAND TO PEAT (A-8)
	GRAY TO BROWN SILT TO CLAY (A-7-6)
	CALCAREOUS CLAY TO WEATHERED LIMESTONE
	AASHTO GROUP SYMBOL AS DETERMINED BY VISUAL REVIEW AND LABORATORY TESTING ON SELECTED SAMPLES FOR CONFIRMATION OF VISUAL REVIEW.
	NUMBERS TO THE LEFT OF BORINGS INDICATE SPT VALUE FOR 12 INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).
	NUMBER OF BLOWS FOR 4 INCHES OF PENETRATION
	HAND AUGERED TO VERIFY UTILITY CLEARANCE
	PERCENT PASSING #200 SIEVE NATURAL MOISTURE CONTENT (%) LIQUID LIMIT (%) PLASTICITY INDEX (%) ORGANIC CONTENT (%) NON-PLASTIC
8	NORTH AMERICAN VERTICAL DATUM OF 1988
	APPROXIMATE SPT BORING LOCATION
	APPROXIMATE AUGER BORING LOCATION
	APPROXIMATE DRIT LOCATION
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE AT OR ABOVE GRADE
	GROUNDWATER LEVEL ENCOUNTERED DURING FIELD EXPLORATIONS
	GROUNDWATER LEVEL ENCOUNTERED AT OR ABOVE GRADE DURING FIELD EXPLORATIONS
1	BORING TERMINATED DUE TO CAVE-IN FROM GROUNDWATER INTRUSION

	SAFETY HAMMER	AUTOMATIC HAMMER
NULAR MATERIALS-	SPT N-VALUE	SPT N-VALUE
ATIVE DENSITY	(BLOWS/FT.)	(BLOWS/FT.)
Y LOOSE	LESS THAN 4	LESS THAN 3
SE	4 to 10	3 to 8
IUM DENSE	10 to 30	8 to 24
SE	30 to 50	24 to 40
Y DENSE	GREATER THAN 50	GREATER THAN 40
S AND CLAYS	SPT N-VALUE	SPT N-VALUE
ONSISTENCY	(BLOWS/FT.)	(BLOWS/FT.)
Y SOFT	LESS THAN 2	LESS THAN 1
T	2 to 4	1 to 3
1	4 to 8	3 to 6
F	8 to 15	6 to 12
Y STIFF	15 to 30	12 to 24
D	GREATER THAN 30	GREATER THAN 24

POND SOIL SURVEY (5)

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LEGEND	

	LEGEND
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT (A-3)
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND (A-2-4)
	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYEY SAND (A-2-4)
	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY (A-2-6/A-6/A-4)
	DARK GRAY TO BLACK ORGANIC SAND TO ORGANIC SILTY SAND TO PEAT (A-8)
	GRAY TO BROWN SILT TO CLAY (A-7-6)
	CALCAREOUS CLAY TO WEATHERED LIMESTONE
	AASHTO GROUP SYMBOL AS DETERMINED BY VISUAL REVIEW AND LABORATORY TESTING ON SELECTED SAMPLES FOR CONFIRMATION OF VISUAL REVIEW.
	NUMBERS TO THE LEFT OF BORINGS INDICATE SPT VALUE FOR 12 INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).
	NUMBER OF BLOWS FOR 4 INCHES OF PENETRATION
	HAND AUGERED TO VERIFY UTILITY CLEARANCE
	PERCENT PASSING #200 SIEVE NATURAL MOISTURE CONTENT (%) LIQUID LIMIT (%) PLASTICITY INDEX (%) ORGANIC CONTENT (%) NON-PLASTIC
8	NORTH AMERICAN VERTICAL DATUM OF 1988
	APPROXIMATE SPT BORING LOCATION
	APPROXIMATE AUGER BORING LOCATION
	APPROXIMATE DRIT LOCATION
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE AT OR ABOVE GRADE
	GROUNDWATER LEVEL ENCOUNTERED DURING FIELD EXPLORATIONS
	GROUNDWATER LEVEL ENCOUNTERED AT OR ABOVE GRADE DURING FIELD EXPLORATIONS
/	BORING TERMINATED DUE TO CAVE-IN FROM GROUNDWATER INTRUSION

	SAFETY HAMMER	AUTOMATIC HAMMER
NULAR MATERIALS-	SPT N-VALUE	SPT N-VALUE
ATIVE DENSITY	(BLOWS/FT.)	(BLOWS/FT.)
Y LOOSE	LESS THAN 4	LESS THAN 3
SE	4 to 10	3 to 8
DIUM DENSE	10 to 30	8 to 24
SE	30 to 50	24 to 40
Y DENSE	GREATER THAN 50	GREATER THAN 40
S AND CLAYS	SPT N-VALUE	SPT N-VALUE
ONSISTENCY	(BLOWS/FT.)	(BLOWS/FT.)
Y SOFT T 1 F F Y STIFF D	LESS THAN 2 2 to 4 4 to 8 8 to 15 15 to 30 GREATER THAN 30	LESS THAN 1 1 to 3 3 to 6 6 to 12 12 to 24 GREATER THAN 24

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#### POND SOIL SURVEY (6)



LEGEND	

	LEGEND
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT (A-3)
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND (A-2-4)
	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYEY SAND (A-2-4)
	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY (A-2-6/A-6/A-4)
	DARK GRAY TO BLACK ORGANIC SAND TO ORGANIC SILTY SAND TO PEAT (A-8)
	GRAY TO BROWN SILT TO CLAY (A-7-6)
	CALCAREOUS CLAY TO WEATHERED LIMESTONE
	AASHTO GROUP SYMBOL AS DETERMINED BY VISUAL REVIEW AND LABORATORY TESTING ON SELECTED SAMPLES FOR CONFIRMATION OF VISUAL REVIEW.
	NUMBERS TO THE LEFT OF BORINGS INDICATE SPT VALUE FOR 12 INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).
	NUMBER OF BLOWS FOR 4 INCHES OF PENETRATION
	HAND AUGERED TO VERIFY UTILITY CLEARANCE
	PERCENT PASSING #200 SIEVE NATURAL MOISTURE CONTENT (%) LIQUID LIMIT (%) PLASTICITY INDEX (%) ORGANIC CONTENT (%) NON-PLASTIC
8	NORTH AMERICAN VERTICAL DATUM OF 1988
	APPROXIMATE SPT BORING LOCATION
	APPROXIMATE AUGER BORING LOCATION
	APPROXIMATE DRIT LOCATION
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE AT OR ABOVE GRADE
	GROUNDWATER LEVEL ENCOUNTERED DURING FIELD EXPLORATIONS
	GROUNDWATER LEVEL ENCOUNTERED AT OR ABOVE GRADE DURING FIELD EXPLORATIONS
1	BORING TERMINATED DUE TO CAVE-IN FROM GROUNDWATER INTRUSION

	SAFETY HAMMER	AUTOMATIC HAMMER
NULAR MATERIALS- ATIVE DENSITY	SPT N-VALUE (BLOWS/FT.)	SPT N-VALUE (BLOWS/FT.)
Y LOOSE	LESS THAN 4	LESS THAN 3
IUM DENSE	10 to 30	8 to 24
SE Y DENSE	<i>30 to 50</i> GREATER THAN 50	24 to 40 GREATER THAN 40
S AND CLAYS ONSISTENCY	SPT N-VALUE (BLOWS/FT.)	SPT N-VALUE (BLOWS/FT.)
Y SOFT	LESS THAN 2	LESS THAN 1
Т	2 to 4	1 to 3
1	4 to 8	3 to 6
F	8 to 15	6 to 12
Y STIFF	15 to 30	12 to 24
D	GREATER THAN 30	GREATER THAN 24
D	GREATER THAN 30	GREATER THAN 24

POND SOIL SURVEY (7)

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LEGEND	

	LEGEND
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT (A-3)
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND (A-2-4)
	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYEY SAND (A-2-4)
	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY (A-2-6/A-6/A-4)
	DARK GRAY TO BLACK ORGANIC SAND TO ORGANIC SILTY SAND TO PEAT (A-8)
	GRAY TO BROWN SILT TO CLAY (A-7-6)
	CALCAREOUS CLAY TO WEATHERED LIMESTONE
	AASHTO GROUP SYMBOL AS DETERMINED BY VISUAL REVIEW AND LABORATORY TESTING ON SELECTED SAMPLES FOR CONFIRMATION OF VISUAL REVIEW.
	NUMBERS TO THE LEFT OF BORINGS INDICATE SPT VALUE FOR 12 INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).
	NUMBER OF BLOWS FOR 4 INCHES OF PENETRATION
	HAND AUGERED TO VERIFY UTILITY CLEARANCE
	PERCENT PASSING #200 SIEVE NATURAL MOISTURE CONTENT (%) LIQUID LIMIT (%) PLASTICITY INDEX (%) ORGANIC CONTENT (%) NON-PLASTIC
8	NORTH AMERICAN VERTICAL DATUM OF 1988
	APPROXIMATE SPT BORING LOCATION
	APPROXIMATE AUGER BORING LOCATION
	APPROXIMATE DRIT LOCATION
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE AT OR ABOVE GRADE
	GROUNDWATER LEVEL ENCOUNTERED DURING FIELD EXPLORATIONS
	GROUNDWATER LEVEL ENCOUNTERED AT OR ABOVE GRADE DURING FIELD EXPLORATIONS
I	BORING TERMINATED DUE TO CAVE-IN FROM GROUNDWATER INTRUSION

	SAFETY HAMMER	AUTOMATIC HAMMER
NULAR MATERIALS-	SPT N-VALUE	SPT N-VALUE
ATIVE DENSITY	(BLOWS/FT.)	(BLOWS/FT.)
Y LOOSE	LESS THAN 4	LESS THAN 3
SE	4 to 10	3 to 8
IUM DENSE	10 to 30	8 to 24
SE	30 to 50	24 to 40
Y DENSE	GREATER THAN 50	GREATER THAN 40
S AND CLAYS	SPT N-VALUE	SPT N-VALUE
ONSISTENCY	(BLOWS/FT.)	(BLOWS/FT.)
Y SOFT	LESS THAN 2	LESS THAN 1
T	2 to 4	1 to 3
1	4 to 8	3 to 6
SF	8 to 15	6 to 12
Y STIFF	15 to 30	12 to 24
D	GREATER THAN 30	GREATER THAN 24

SHEET NUMBER

#### POND SOIL SURVEY (8)



LEGEND	

	LEGEND
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT (A-3)
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND (A-2-4)
	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYEY SAND (A-2-4)
	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY (A-2-6/A-6/A-4)
	DARK GRAY TO BLACK ORGANIC SAND TO ORGANIC SILTY SAND TO PEAT (A-8)
	GRAY TO BROWN SILT TO CLAY (A-7-6)
	CALCAREOUS CLAY TO WEATHERED LIMESTONE
	AASHTO GROUP SYMBOL AS DETERMINED BY VISUAL REVIEW AND LABORATORY TESTING ON SELECTED SAMPLES FOR CONFIRMATION OF VISUAL REVIEW.
	NUMBERS TO THE LEFT OF BORINGS INDICATE SPT VALUE FOR 12 INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).
	NUMBER OF BLOWS FOR 4 INCHES OF PENETRATION
	HAND AUGERED TO VERIFY UTILITY CLEARANCE
	PERCENT PASSING #200 SIEVE NATURAL MOISTURE CONTENT (%) LIQUID LIMIT (%) PLASTICITY INDEX (%) ORGANIC CONTENT (%) NON-PLASTIC
8	NORTH AMERICAN VERTICAL DATUM OF 1988
	APPROXIMATE SPT BORING LOCATION
	APPROXIMATE AUGER BORING LOCATION
	APPROXIMATE DRIT LOCATION
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE AT OR ABOVE GRADE
	GROUNDWATER LEVEL ENCOUNTERED DURING FIELD EXPLORATIONS
	GROUNDWATER LEVEL ENCOUNTERED AT OR ABOVE GRADE DURING FIELD EXPLORATIONS
/	BORING TERMINATED DUE TO CAVE-IN FROM GROUNDWATER INTRUSION

	SAFETY HAMMER	AUTOMATIC HAMMER
NULAR MATERIALS-	SPT N-VALUE	SPT N-VALUE
ATIVE DENSITY	(BLOWS/FT.)	(BLOWS/FT.)
Y LOOSE	LESS THAN 4	LESS THAN 3
SE	4 to 10	3 to 8
IUM DENSE	10 to 30	8 to 24
SE	30 to 50	24 to 40
Y DENSE	GREATER THAN 50	GREATER THAN 40
S AND CLAYS	SPT N-VALUE	SPT N-VALUE
ONSISTENCY	(BLOWS/FT.)	(BLOWS/FT.)
Y SOFT	LESS THAN 2	LESS THAN 1
T	2 to 4	1 to 3
1	4 to 8	3 to 6
F	8 to 15	6 to 12
Y STIFF	15 to 30	12 to 24
D	GREATER THAN 30	GREATER THAN 24

SHEET NUMBER

#### POND SOIL SURVEY (9)



	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT (A-3)
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND (A-2-4)
	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYEY SAND (A-2-4)
	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY TO SILT (A-2-6/A-6/A-4)
	DARK GRAY TO BLACK ORGANIC SAND TO ORGANIC SILTY SAND TO PEAT (A-8)
	GRAY TO BROWN SILT TO CLAY (A-7-6)
	CALCAREOUS CLAY TO WEATHERED LIMESTONE
	LANDFILL DEBRIS
	AASHTO GROUP SYMBOL AS DETERMINED BY VISUAL REVIEW AND LABORATORY TESTING ON SELECTED SAMPLES FOR CONFIRMATION OF VISUAL REVIEW.
	NUMBERS TO THE LEFT OF BORINGS INDICATE SPT VALUE FOR 12 INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).
	HAND AUGERED TO VERIFY UTILITY CLEARANCE
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE AT OR ABOVE GRADE
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE
	GROUNDWATER LEVEL ENCOUNTERED DURING FIELD EXPLORATIONS
	GROUNDWATER LEVEL ENCOUNTERED AT OR ABOVE GRADE DURING FIELD EXPLORATIONS
	GROUNDWATER LEVEL NOT RECORDED
1	CAVE-IN DUE TO SHALLOW GROUNDWATER INTRUSION
L	REFUSAL DUE TO HAND AUGER REFUSAL ON LANDFILL DEBRIS
Τ.	CENTERLINE CONSTRUCTION LENA ROAD
1.	THE BORINGS WERE LOCATED IN THE FIELD BY A

I. THE BORINGS WERE LOCATED IN THE FIELD BY REPRESENTATIVE OF TIERRA USING A HAND-HELD GARMIN ETREX GPS UNIT. THE BORINGS INDICATED WITH AN "\*" WERE LATER SURVEY LOCATED BY THE PROJECT SURVEYOR. STATION AND OFFSET FOR THE BORINGS WERE DETERMINED BY TIERRA USING THE FROM FIELD GPS AND THE SURVEYED COORDINATES IN CONJUNCTION WITH PROJECT DESIGN FILES PROVIDED BY KIMLEY HORN.

	SAFETY HAMMER	AUTOMATIC HAMMER
ULAR MATERIALS-	SPT N-VALUE	SPT N-VALUE
ATIVE DENSITY	(BLOWS/FT.)	(BLOWS/FT.)
' LOOSE	LESS THAN 4	LESS THAN 3
5E	4 to 10	3 to 8
IUM DENSE	10 to 30	8 to 24
5E	30 to 50	24 to 40
' DENSE	GREATER THAN 50	GREATER THAN 40
S AND CLAYS	SPT N-VALUE	SPT N-VALUE
DNSISTENCY	(BLOWS/FT.)	(BLOWS/FT.)
SOFT STIFF	LESS THAN 2 2 to 4 4 to 8 8 to 15 15 to 30 GREATER THAN 30	LESS THAN 1 1 to 3 3 to 6 6 to 12 12 to 24 GREATER THAN 24

SHEET NUMBER

# DEBRIS SOIL PROFILES (1)



	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT (A-3)
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND (A-2-4)
	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYEY SAND (A-2-4)
	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY TO SILT (A-2-6/A-6/A-4)
	DARK GRAY TO BLACK ORGANIC SAND TO ORGANIC SILTY SAND TO PEAT (A-8)
	GRAY TO BROWN SILT TO CLAY (A-7-6)
	CALCAREOUS CLAY TO WEATHERED LIMESTONE
	LANDFILL DEBRIS
	AASHTO GROUP SYMBOL AS DETERMINED BY VISUAL REVIEW AND LABORATORY TESTING ON SELECTED SAMPLES FOR CONFIRMATION OF VISUAL REVIEW.
	NUMBERS TO THE LEFT OF BORINGS INDICATE SPT VALUE FOR 12 INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).
	HAND AUGERED TO VERIFY UTILITY CLEARANCE
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE AT OR ABOVE GRADE
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE
	GROUNDWATER LEVEL ENCOUNTERED DURING FIELD EXPLORATIONS
	GROUNDWATER LEVEL ENCOUNTERED AT OR ABOVE GRADE DURING FIELD EXPLORATIONS
	GROUNDWATER LEVEL NOT RECORDED
1	CAVE-IN DUE TO SHALLOW GROUNDWATER INTRUSION
L	REFUSAL DUE TO HAND AUGER REFUSAL ON LANDFILL DEBRIS
т.	CENTERLINE CONSTRUCTION LENA ROAD
1.	THE BORINGS WERE LOCATED IN THE FIELD BY A

I. THE BORINGS WERE LOCATED IN THE FIELD BY A REPRESENTATIVE OF TIERRA USING A HAND-HELD GARMIN ETREX GPS UNIT. THE BORINGS INDICATED WITH AN "\*" WERE LATER SURVEY LOCATED BY THE PROJECT SURVEYOR. STATION AND OFFSET FOR THE BORINGS WERE DETERMINED BY TIERRA USING THE FROM FIELD GPS AND THE SURVEYED COORDINATES IN CONJUNCTION WITH PROJECT DESIGN FILES PROVIDED BY KIMLEY HORN.

	SAFETY HAMMER	AUTOMATIC HAMMER
ULAR MATERIALS-	SPT N-VALUE	SPT N-VALUE
ATIVE DENSITY	(BLOWS/FT.)	(BLOWS/FT.)
' LOOSE	LESS THAN 4	LESS THAN 3
5E	4 to 10	3 to 8
IUM DENSE	10 to 30	8 to 24
5E	30 to 50	24 to 40
' DENSE	GREATER THAN 50	GREATER THAN 40
S AND CLAYS	SPT N-VALUE	SPT N-VALUE
DNSISTENCY	(BLOWS/FT.)	(BLOWS/FT.)
SOFT STIFF	LESS THAN 2 2 to 4 4 to 8 8 to 15 15 to 30 GREATER THAN 30	LESS THAN 1 1 to 3 3 to 6 6 to 12 12 to 24 GREATER THAN 24

SHEET NUMBER

## DEBRIS SOIL PROFILES (2)



	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT (A-3)
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND (A-2-4)
	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYEY SAND (A-2-4)
	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY TO SILT (A-2-6/A-6/A-4)
	DARK GRAY TO BLACK ORGANIC SAND TO ORGANIC SILTY SAND TO PEAT (A-8)
	GRAY TO BROWN SILT TO CLAY (A-7-6)
	CALCAREOUS CLAY TO WEATHERED LIMESTONE
	LANDFILL DEBRIS
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	NUMBERS TO THE LEFT OF BORINGS INDICATE SPT VALUE FOR 12 INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).
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	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE AT OR ABOVE GRADE
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE
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	GROUNDWATER LEVEL ENCOUNTERED AT OR ABOVE GRADE DURING FIELD EXPLORATIONS
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	SAFETY HAMMER	AUTOMATIC HAMMER
ULAR MATERIALS-	SPT N-VALUE	SPT N-VALUE
ATIVE DENSITY	(BLOWS/FT.)	(BLOWS/FT.)
' LOOSE	LESS THAN 4	LESS THAN 3
5E	4 to 10	3 to 8
IUM DENSE	10 to 30	8 to 24
5E	30 to 50	24 to 40
' DENSE	GREATER THAN 50	GREATER THAN 40
S AND CLAYS	SPT N-VALUE	SPT N-VALUE
DNSISTENCY	(BLOWS/FT.)	(BLOWS/FT.)
SOFT STIFF	LESS THAN 2 2 to 4 4 to 8 8 to 15 15 to 30 GREATER THAN 30	LESS THAN 1 1 to 3 3 to 6 6 to 12 12 to 24 GREATER THAN 24

SHEET NUMBER

## DEBRIS SOIL PROFILES (3)



	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT (A-3)
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND (A-2-4)
	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYEY SAND (A-2-4)
	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY TO SILT (A-2-6/A-6/A-4)
	DARK GRAY TO BLACK ORGANIC SAND TO ORGANIC SILTY SAND TO PEAT (A-8)
	GRAY TO BROWN SILT TO CLAY (A-7-6)
	CALCAREOUS CLAY TO WEATHERED LIMESTONE
	LANDFILL DEBRIS
	AASHTO GROUP SYMBOL AS DETERMINED BY VISUAL REVIEW AND LABORATORY TESTING ON SELECTED SAMPLES FOR CONFIRMATION OF VISUAL REVIEW.
	NUMBERS TO THE LEFT OF BORINGS INDICATE SPT VALUE FOR 12 INCHES OF PENETRATION (UNLESS OTHERWISE NOTED).
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	GROUNDWATER LEVEL ENCOUNTERED DURING FIELD EXPLORATIONS
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	SAFETY HAMMER	AUTOMATIC HAMMER
ULAR MATERIALS-	SPT N-VALUE	SPT N-VALUE
ATIVE DENSITY	(BLOWS/FT.)	(BLOWS/FT.)
' LOOSE	LESS THAN 4	LESS THAN 3
5E	4 to 10	3 to 8
IUM DENSE	10 to 30	8 to 24
5E	30 to 50	24 to 40
' DENSE	GREATER THAN 50	GREATER THAN 40
S AND CLAYS	SPT N-VALUE	SPT N-VALUE
DNSISTENCY	(BLOWS/FT.)	(BLOWS/FT.)
SOFT STIFF	LESS THAN 2 2 to 4 4 to 8 8 to 15 15 to 30 GREATER THAN 30	LESS THAN 1 1 to 3 3 to 6 6 to 12 12 to 24 GREATER THAN 24

SHEET NUMBER

## DEBRIS SOIL PROFILES (4)



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#### IEGEND

	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT (A-3)
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND (A-2-4)
	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYEY SAND (A-2-4)
	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY TO SILT (A-2-6/A-6/A-4)
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L	REFUSAL DUE TO HAND AUGER REFUSAL ON LANDFILL DEBRIS
Т.	CENTERLINE CONSTRUCTION LENA ROAD
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	SAFETY HAMMER	AUTOMATIC HAMMER
ULAR MATERIALS-	SPT N-VALUE	SPT N-VALUE
ATIVE DENSITY	(BLOWS/FT.)	(BLOWS/FT.)
' LOOSE	LESS THAN 4	LESS THAN 3
5E	4 to 10	3 to 8
IUM DENSE	10 to 30	8 to 24
5E	30 to 50	24 to 40
' DENSE	GREATER THAN 50	GREATER THAN 40
S AND CLAYS	SPT N-VALUE	SPT N-VALUE
DNSISTENCY	(BLOWS/FT.)	(BLOWS/FT.)
SOFT STIFF	LESS THAN 2 2 to 4 4 to 8 8 to 15 15 to 30 GREATER THAN 30	LESS THAN 1 1 to 3 3 to 6 6 to 12 12 to 24 GREATER THAN 24

SHEET NUMBER

# DEBRIS SOIL PROFILES (5)



	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SAND TO SAND WITH SILT (A-3)
	BROWN TO LIGHT BROWN TO GRAY TO LIGHT GRAY SILTY SAND (A-2-4)
	BROWN TO LIGHT BROWN TO GRAY SILTY SAND TO SILTY-CLAYEY SAND (A-2-4)
	GRAY TO BROWN CLAYEY SAND TO SANDY CLAY TO SILT (A-2-6/A-6/A-4)
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	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE AT OR ABOVE GRADE
	ESTIMATED SEASONAL HIGH GROUNDWATER TABLE
	GROUNDWATER LEVEL ENCOUNTERED DURING FIELD EXPLORATIONS
	GROUNDWATER LEVEL ENCOUNTERED AT OR ABOVE GRADE DURING FIELD EXPLORATIONS
	GROUNDWATER LEVEL NOT RECORDED
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Τ.	CENTERLINE CONSTRUCTION LENA ROAD
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	SAFETY HAMMER	AUTOMATIC HAMMER
ULAR MATERIALS-	SPT N-VALUE	SPT N-VALUE
ATIVE DENSITY	(BLOWS/FT.)	(BLOWS/FT.)
' LOOSE	LESS THAN 4	LESS THAN 3
5E	4 to 10	3 to 8
IUM DENSE	10 to 30	8 to 24
5E	30 to 50	24 to 40
' DENSE	GREATER THAN 50	GREATER THAN 40
S AND CLAYS	SPT N-VALUE	SPT N-VALUE
DNSISTENCY	(BLOWS/FT.)	(BLOWS/FT.)
SOFT STIFF	LESS THAN 2 2 to 4 4 to 8 8 to 15 15 to 30 GREATER THAN 30	LESS THAN 1 1 to 3 3 to 6 6 to 12 12 to 24 GREATER THAN 24

SHEET NUMBER

# DEBRIS SOIL PROFILES (6)



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# **APPENDIX C**

Summary of Seasonal High Groundwater Table Estimates for Roadway Summary of Seasonal High Groundwater Table Estimates for Ponds Summary of Groundwater Table Measurements from Piezometers

#### Summary of Seasonal High Groundwater Table Estimates for Roadway Lena Road from North of 44th Avenue East to SR 64 Manatee County, Florida Manatee County Project #6107560 Tierra Project No.: 6511-22-127

		<b>Devine</b> L	(1)					Measure	ed	USD	A Soil Survey	Es	timated
Boring		Boring Lo			Boring	Ground	G	roundwate	r Table		Estimated	Sł	IGWT <sup>(5)</sup>
Name	Station (feet)	Offset (feet)	FL Wes Easting (feet)	t NAD 83 Northing (feet)	(feet)	(feet, NAVD 88)	Date Recorded	Depth <sup>(3)</sup> (feet)	Elevation (feet, NAVD 88)	Map Symbol	SHGWT <sup>(4)</sup> Depth (feet)	Depth <sup>(3)</sup> (feet)	Elevation (feet, NAVD 88)
SH-219	219+20	13' LT.	508655	1137310	5.0	23.1	7/11/2022	1.8	21.3	20	0.3-1.5	1.0	22.1
SH-222	222+08	5' LT.	508665	1137598	5.0	30.8	7/11/2022	2.9	27.9	20	0.3-1.5	1.5	29.3
SH-225	224+79	24' LT.	508648	1137869	5.0	31.1	7/11/2022	1.5	29.6	20	0.3-1.5	1.3	29.8
SH-228	227+77 <sup>(7)</sup>	30' RT. <sup>(7)</sup>	508686 <sup>(7)</sup>	1138173 <sup>(7)</sup>	4.0	28.8 <sup>(7)</sup>	7/11/2022	0.8	28.0	26	+2.0-0.0	ABG <sup>(6)</sup>	>28.8
SH-232	232+16	24' LT.	508418	1138525	5.0	29.6	7/11/2022	2.5	27.1	26	+2.0-0.0	1.0	28.6
SH-234	233+59 <sup>(7)</sup>	38' RT. <sup>(7)</sup>	508377 <sup>(7)</sup>	1138676 <sup>(7)</sup>	4.0	28.5 <sup>(7)</sup>	7/11/2022	3.0	25.5	26	+2.0-0.0	1.5	27.0
SH-237	237+10 <sup>(7)</sup>	1' LT. <sup>(7)</sup>	508129 <sup>(7)</sup>	1138927 <sup>(7)</sup>	5.0	28.5 <sup>(7)</sup>	7/11/2022	0.7	27.8	26	+2.0-0.0	ABG <sup>(6)</sup>	>28.5
SH-240	239+94	14' RT.	507972	1139162	5.0	30.9	7/11/2022	3.1	27.8	20	0.3-1.5	1.5	29.4
SH-243	242+94	28' LT.	507849	1139439	5.0	30.2	7/6/2022	2.6	27.6	20	0.3-1.5	0.5	29.7
SH-246	245+83	17' RT.	507897	1139729	5.0	32.5	7/6/2022	4.4	28.1	20	0.3-1.5	2.5	30.0
SH-249	248+99	26' LT.	507858	1140045	5.0	32.0	7/6/2022	4.6	27.4	12	3.5-5.0	2.5	29.5
SH-252	252+29	18' RT.	507933	1140368	5.5	32.9	7/6/2022	5.3	27.6	20	0.3-1.5	3.5	29.4
SH-255	255+35	28' LT.	508001	1140668	5.0	30.9	7/6/2022	3.1	27.8	20	0.3-1.5	1.5	29.4
SH-258	257+79	47' RT.	508141	1140884	5.0	31.2	7/6/2022	4.4	26.8	20	0.3-1.5	2.0	29.2
SH-261	261+35	29' LT.	508080	1141247	5.0	30.2	7/6/2022	4.1	26.1	20	0.3-1.5	2.0	28.2
SH-264	263+87	25' RT.	508140	1141498	5.5	29.6	7/6/2022	4.2	25.4	35	0.0-1.5	2.0	27.6
SH-267	266+74	15' LT.	508106	1141785	6.0	30.1	7/6/2022	5.3	24.8	35	0.0-1.5	2.5	27.6
SH-270	269+90	23' RT.	508152	1142101	6.0	28.7	7/6/2022	5.8	22.9	11	1.5-3.5	2.0	26.7

<sup>1)</sup> The boring locations and ground elevations were provided by the project surveyor unless otherwise noted. Project design files were utilized in conjunction with the coordinates provided by the project surveyor to determine the station and offset of the boring locations. Station and Offset are referenced to the Centerline Construction Lena Road alignment.

<sup>2)</sup> Depth of boring below existing grades. Shallow borings less than 5 feet in depth caved in due to groundwater intrusion.

<sup>3)</sup> Depth below grade at time of boring.

<sup>4)</sup> Seasonal high groundwater table depth based on the Manatee County, Florida USDA Soil Survey information.

<sup>5)</sup> Seasonal high groundwater table depth estimated based on soil stratigraphy, measured groundwater levels from the borings, the Manatee County, Florida USDA Soil Survey information, and past experience with similar soil conditions in the project area.

<sup>(6)</sup> We recommend the project biologist be consulted to assist with determining seasonal high groundwater table levels using biological indicators and/or available wetland information at these locations.

<sup>7)</sup> These borings were not survey located. The boring locations were determined in the field using hand-held, non-survey grade GPS equipment with a manufacturer's reported accuracy of ± 10 feet. Station, offset and elevation of the boring locations were determined using the GPS coordinates recorded in the field in conjunction with project design files provided by Kimley-Horn and Associates. The boring locations and elevations should be considered approximate.

ABG: At or Above Existing Grade

#### Summary of Seasonal High Groundwater Table Estimates for Roadway Lena Road from North of 44th Avenue East to SR 64 Manatee County, Florida Manatee County Project #6107560 Tierra Project No.: 6511-22-127

		Poring L	$\mathbf{n}$					Measure	ed	USD	A Soil Survey	Es	timated
Boring		воппу с			Boring	Ground	G	roundwate	r Table		Estimated	SH	IGWT <sup>(5)</sup>
Name	Station (feet)	Offset (feet)	FL Wes Easting (feet)	t NAD 83 Northing (feet)	l Depth <sup>(-)</sup> (feet)	Elevation <sup>(*)</sup> (feet, NAVD 88)	Date Recorded	Depth <sup>(3)</sup> (feet)	Elevation (feet, NAVD 88)	Map Symbol	SHGWT <sup>(4)</sup> Depth (feet)	Depth <sup>(3)</sup> (feet)	Elevation (feet, NAVD 88)
SH-273	272+98	10' LT.	508124	1142409	8.0	29.1	7/6/2022	5.5	23.6	45	1.5-3.5	2.5	26.6
SH-276	276+40	25' RT.	508123	1142753	7.0	27.2	7/8/2022	6.5	20.7	11	1.5-3.5	2.5	24.7
SH-279	278+94	22' LT.	508022	1142992	5.0	25.5	7/11/2022	4.4	21.1	11	1.5-3.5	2.0	23.5
SH-282	281+94	11' RT.	508003	1143293	5.0	23.9	7/8/2022	4.5	19.4	11	1.5-3.5	2.0	21.9
SH-285	284+93	38' LT.	507953	1143592	6.0	21.2	7/11/2022	4.6	16.6	20	0.3-1.5	2.0	19.2
SH-288	287+95	7' RT.	508011	1143891	5.0	22.1	7/8/2022	4.5	17.6	20	0.3-1.5	3.0	19.1
SH-291	290+95	38' LT.	507980	1144193	6.0	20.6	7/11/2022	4.2	16.4	20	0.3-1.5	2.0	18.6
SH-294	293+90	24' RT.	508054	1144485	5.5	18.8	7/8/2022	4.0	14.8	20	0.3-1.5	1.5	17.3
SH-297	296+89	37' LT.	508006	1144787	5.0	19.0	7/11/2022	3.6	15.4	20	0.3-1.5	1.3	17.7
SH-300	299+92	20' RT.	508076	1145087	5.0	19.3	7/8/2022	2.5	16.8	20	0.3-1.5	2.0	17.3
SH-303	303+03	39' LT.	508027	1145400	5.0	19.7	7/8/2022	4.0	15.7	20	0.3-1.5	2.5	17.2
SH-306	305+96	25' RT.	508094	1145692	5.0	18.6	7/8/2022	3.5	15.1	20	0.3-1.5	2.0	16.6
SH-309	309+05	35' LT.	508036	1146003	5.0	18.9	7/8/2022	4.0	14.9	20	0.3-1.5	2.5	16.4
SH-FPC-1	230+20 <sup>(7)</sup>	186' LT. <sup>(7)</sup>	508407 <sup>(7)</sup>	1138276 <sup>(7)</sup>	5.0	30.3 <sup>(7)</sup>	7/11/2022	3.0	27.3	26	+2.0-0.0	1.5	28.8
SH-FPC-2	285+55 <sup>(7)</sup>	419' RT. <sup>(7)</sup>	508413 <sup>(7)</sup>	1143634 <sup>(7)</sup>	5.0	22.8 <sup>(7)</sup>	7/21/2022	4.8	18.0	20	0.3-1.5	1.5	21.3
SH-POND1-1	250+52	190' RT.	508076	1140184	6.0	32.3	7/6/2022	4.8	27.5	20	0.3-1.5	3.0	29.3
SH-ROUNDABOUT-1	302+14	54' LT.	508011	1145310	5.0	19.5	7/11/2022	3.3	16.2	20	0.3-1.5	2.0	17.5
SH-ROUNDABOUT-2	302+33	73' RT.	508138	1145328	5.0	19.0	7/11/2022	2.5	16.5	20	0.3-1.5	1.5	17.5

The boring locations and ground elevations were provided by the project surveyor unless otherwise noted. Project design files were utilized in conjunction with the coordinates provided by the project surveyor to determine the station and offset of the boring locations. Station and Offset are referenced to the Centerline Construction Lena Road alignment.

Depth of boring below existing grades. Shallow borings less than 5 feet in depth caved in due to groundwater intrusion.

Depth below grade at time of boring.

Seasonal high groundwater table depth based on the Manatee County, Florida USDA Soil Survey information.

Seasonal high groundwater table depth estimated based on soil stratigraphy, measured groundwater levels from the borings, the Manatee County, Florida USDA Soil Survey information, and past experience with similar soil conditions in the project area. We recommend the project biologist be consulted to assist with determining seasonal high groundwater table levels using biological indicators and/or available wetland information at these locations.

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using the GPS coordinates recorded in the field in conjunction with project design files provided by Kimley-Horn and Associates. The boring locations and elevations should be considered approximate. ABG: At or Above Existing Grade

#### Summary of Seasonal High Groundwater Table Estimates for Ponds Lena Road from North of 44th Avenue East to SR 64 Manatee County, Florida Manatee County Project #6107560 Tierra Project No.: 6511-22-127

						nena Project N	0 6511-22-127						
		Denin a L	• • • • • • • (1)					Measured		USD	A Soil Survey	Es	timated
Boring		Boring Lo	ocation		Boring	Ground	Grou	undwater Tab	le		Estimated	SH	IGWT <sup>(5)</sup>
Name	Station (feet)	Offset (feet)	FL Wes Easting (feet)	ot NAD 83 Northing (feet)	Depth <sup>(2)</sup> (feet)	Elevation <sup>(1)</sup> (feet, NAVD 88)	Date Recorded	Depth <sup>(3)</sup> (feet)	Elevation (feet, NAVD 88)	Map Symbol	SHGWT <sup>(4)</sup> Depth (feet)	Depth <sup>(3)</sup> (feet)	Elevation (feet, NAVD 88)
HA-FPC-1	235+98	153' RT.	508319	1138934	3.0	28.5	10/30/2022	0.00	28.5	26	+2.0-0.0	ABG <sup>(6)</sup>	28.5
HA-FPC-2	237+61	276' RT.	508315	1139139	4.0	28.2	10/30/2022	0.00	28.2	26	+2.0-0.0	ABG <sup>(6)</sup>	28.2
PBA-3	238+70	277' RT.	508249	1139225	3.0	28.6	10/30/2022	0.30	28.3	26	+2.0-0.0	ABG <sup>(6)</sup>	28.6
PBA-1	238+80	113' RT.	508114	1139131	3.5	28.8	10/30/2022	0.00	28.8	26	+2.0-0.0	ABG <sup>(6)</sup>	28.8
PBA-2	240+57	192' RT.	508107	1139289	5.0	30.9	10/30/2022	3.20	27.7	20/26	0.3-1.5/+2.0-0.0	2.0	28.9
PBA-4	241+56	344' RT.	508231	1139394	4.5	29.8	10/30/2022	1.80	28.0	20/26	0.3-1.5/+2.0-0.0	ABG <sup>(6)</sup>	29.8
HA-FPC-12	280+68	345' RT.	508347	1143225	5.0	24.7	10/12/2022	2.70	22.0	11	1.5-3.5	1.5	23.2
HA-FPC-15	282+76	477' RT.	508463	1143390	5.0	23.8	10/12/2022	2.80	21.0	20	0.3-1.5	1.5	22.3
HA-FPC-13	283+25	380' RT.	508365	1143423	5.0	23.9	10/12/2022	2.70	21.2	20	0.3-1.5	1.5	22.4
HA-FPC-5	283+32	203' RT.	508188	1143429	5.0	23.6	10/12/2022	2.30	21.3	20	0.3-1.5	1.5	22.1
HA-FPC-8	284+17	292' RT.	508279	1143502	5.0	23.4	10/12/2022	2.60	20.8	20	0.3-1.5	1.0	22.4
HA-FPC-3	284+57	116' RT.	508105	1143550	5.0	23.3	10/12/2022	2.50	20.8	20	0.3-1.5	1.5	21.8
HA-FPC-16	284+73	483' RT.	508472	1143549	5.0	23.1	10/12/2022	2.50	20.6	20	0.3-1.5	1.0	22.1
HA-FPC-6	285+22	194' RT.	508186	1143611	5.0	23.0	10/12/2022	2.30	20.7	20	0.3-1.5	1.0	22.0
HA-FPC-9	286+73	281' RT.	508280	1143758	5.0	22.4	10/13/2022	2.80	19.6	20	0.3-1.5	1.5	20.9
HA-FPC-7	286+83	222' RT.	508221	1143770	5.0	22.4	10/13/2022	2.60	19.8	20	0.3-1.5	1.5	20.9
HA-FPC-14	287+54	377' RT.	508380	1143834	5.0	22.0	10/13/2022	2.60	19.4	20	0.3-1.5	0.5	21.5
HA-FPC-4	287+56	113' RT.	508116	1143848	5.0	22.1	10/13/2022	2.70	19.4	20	0.3-1.5	1.5	20.6
HA-FPC-17	287+57	527' RT.	508529	1143832	2.5	21.4	10/12/2022	2.40	19.0	20	0.3-1.5	1.0	20.4
HA-FPC-10	289+50	307' RT.	508318	1144034	3.5	21.5	10/12/2022	2.50	19.0	20	0.3-1.5	1.0	20.5
PBA-5	289+95	163' RT.	508176	1144085	5.0	21.2	10/13/2022	2.80	18.4	20	0.3-1.5	0.5	20.7
PBA-6	291+69	164' RT.	508184	1144259	4.5	20.4	10/13/2022	2.80	17.6	20	0.3-1.5	1.5	18.9
HA-FPC-11	291+92	284' RT.	508306	1144276	5.0	20.4	10/13/2022	3.80	16.6	20	0.3-1.5	2.0	18.4
PBA-7	296+90	76' RT.	508120	1144783	5.0	19.4	10/30/2022	3.50	15.9	20	0.3-1.5	1.5	17.9
PBA-9	300+95	336' RT.	508396	1145176	5.0	19.5	10/13/2022	3.40	16.1	20	0.3-1.5	1.0	18.5
PBA-8	300+96	157' RT.	508218	1145186	5.0	19.3	10/13/2022	2.60	16.7	20	0.3-1.5	1.0	18.3
(1) The bering leastions and ground a	alovationa wara pro	wided by the project		sthemulae noted Dr	alaat daalaa file	a wara utilizad in aaniur	ation with the coordinates pr	ovided by the prei	aat aurupupur ta datarmin	a the station of	and offerst of the hering le	actions	

(1) The boring locations and ground elevations were provided by the project surveyor unless otherwise noted. Project design files were utilized in conjunction with the coordinates provided by the project surveyor to determine the station and offset of the boring locations. Station and Offset are referenced to the Centerline Construction Lena Road alignment.

<sup>(2)</sup> Depth of boring below existing grades. Shallow borings less than 5 feet in depth caved in due to groundwater intrusion.

<sup>(3)</sup> Depth below grade at time of boring.

<sup>(4)</sup> Seasonal high groundwater table depth based on the Manatee County, Florida USDA Soil Survey information.

<sup>5)</sup> Seasonal high groundwater table depth estimated based on soil stratigraphy, measured groundwater levels from the borings, the Manatee County, Florida USDA Soil Survey information, and past experience with similar soil conditions in the project area.

<sup>6)</sup> We recommend the project biologist be consulted to assist with determining seasonal high groundwater table levels using biological indicators and/or available wetland information at these locations.

ABG: At or Above Existing Grade

	Summary of Groundwater Table Measurements from Piezometers Lena Road from North of 44th Avenue East to SR 64 Manatee County, Florida Manatee County Project #6107560													
Manatee County Project #6107560 Tierra Project No.: 6511-22-127														
Piezometer	Piezometer     Piezometer Location <sup>(1)</sup> Ground       Boring     Station     Offset     FL West NAD 83													
Identification	Boring Identification         Station (feet)         Offset (feet)         FL West NAD 63         Elevation <sup>(2)</sup> (feet)         Elevation <sup>(2)</sup> Ture (feet, NAVD 88)           Identification         (feet)         (feet)         (feet)         (feet)         Elevation <sup>(2)</sup> 08/15/22         11/16/22         Future Measurements													
PZ-1	222+09	4' LT.	508666	1137599	29.8	27.2	27.6	27.4						
PZ-2	232+16	22' LT.	508418	1138526	29.8	27.8	28.8	29.3						
PZ-3	242+94	30' LT.	507847	1139438	30.3	29.0	29.1	29.7						
PZ-4	252+31	16' RT.	507932	1140370	33.0	28.5	28.6	29.1						
PZ-5	261+36	29' LT.	508080	1141247	30.5	27.4	27.5	28.5						
PZ-6	273+00	11' LT.	508123	1142410	28.7	22.5	22.7	24.2	-					
PZ-7	281+94	13' RT.	508004	1143293	24.1	20.2	19.9	21.7	-					
PZ-8	290+96	39' LT.	507978	1144194	21.4	17.6	17.9	19.1	-					
PZ-9	303+05	39' LT.	508027	1145401	20.1	17.0	16.6	17.7						
PZ-10	309+05	38' LT.	508034	1146002	18.7	16.1	15.5	15.8						
PZ-POND1-1	250+54	190' RT.	508077	1140186	32.7	28.7	28.5	29.1						

<sup>1)</sup> The boring locations and ground elevations were provided by the project surveyor. Project design files were utilized in conjunction with the coordinates provided by the project surveyor to determine the station and offset of the boring locations. Station and Offset are referenced to the Centerline Construction Lena Road alignment.

<sup>(2)</sup> Ground elevations were provided by the project surveyor.

<sup>(3)</sup> Groundwater elevations calculated by subtracting the depth to the groundwater from the surveyed ground elevation.

# **APPENDIX D**

Summary of Laboratory Test Results for Soil Classification Summary of Laboratory Test Results for Environmental Classification Hydraulic Conductivity Test Results Design LBR and MR Summary Table Results of Limerock Bearing Ratio Test

Summary of Laboratory Test Results for Soil Classification														
Lena Road from North of 44th Avenue East to SR 64														
Manatee County, Florida														
Manatee County Project #6107560														
Boring Name	Sample Depth	Stratum	AASHTO			% Finer	, 1		Atte	rberg Li	mits	Organic	Moisture	
	(feet)			#10	#40	#60	#100	#200	LL	PL	PI	Content	Content	
LBR-5	1.0 - 2.0	1	A-3	100	91	69	34	2	-	-	-	-	-	
LBR-3	1.0 - 2.0	1	A-3	100	93	72	36	3	-	-	-	-	-	
FPC-6	6.0 - 8.0	1	A-3	-	-	-	-	3	-	-	-	-	-	
SH-309	4.0 - 4.5	1	A-3	100	91	72	34	3	-	-	-	-	-	
LBR-4	1.0 - 2.0	1	A-3	100	92	71	33	3	-	-	-	-	-	
FPC-8	8.0 - 10.0	1	A-3	-	-	-	-	3	-	-	-	-	-	
PBA-6	2.5 - 3.0	1	A-3	100	92	73	35	4	-	-	-	-	-	
SH-276	2.5 - 3.0	1	A-3	-	-	-	-	4	-	-	-	-	-	
SH-264	0.5 - 1.5	1	A-3	-	-	-	-	4	-	-	-	-	-	
SH-270	0.5 - 1.0	1	A-3	-	-	-	-	4	-	-	-	-	-	
FPC-3	8.0 - 10.0	1	A-3	-	-	-	-	4	-	-	-	-	-	
LBR-1	1.0 - 2.0	1	A-3	100	93	72	35	4	-	-	-	-	-	
PBA-5	0.5 - 1.0	1	A-3	-	-	-	-	5	-	-	-	2	19	
SH-258	2.0 - 2.5	1	A-3	-	-	-	-	5	-	-	-	-	-	
SH-POND1-1	4.5 - 5.0	1	A-3	100	95	77	40	5	-	-	-	-	-	
SH-219	0.0 - 1.0	1	A-3	-	-	-	-	5	-	-	-	-	-	
LBR-7	1.0 - 2.0	1	A-3	82	72	54	26	5	-	-	-	-	-	
SH-264	3.0 - 3.5	1	A-3	-	-	-	-	5	-	-	-	-	-	
SH-294	0.5 - 1.5	1	A-3	-	-	-	-	5	-	-	-	-	-	
B-293L	6.0 - 8.0	1	A-3	-	-	-	-	5	-	-	-	-	-	
LBR-8	1.0 - 2.0	1	A-3	96	85	65	31	5	-	-	-	-	-	
SH-267	1.0 - 1.5	1	A-3	-	-	-	-	5	-	-	-	-	-	
SH-294	1.5 - 2.0	1	A-3	100	90	71	34	5	-	-	-	-	-	
HA-FPC-16	1.5 - 2.5	1	A-3	100	89	67	35	5	-	-	-	-	-	
LBR-9	1.0 - 2.0	1	A-3	95	87	68	35	5	-	-	-	-	-	
SH-252	3.5 - 4.0	1	A-3	-	-	-	-	5	-	-	-	-	-	
LBR-6	1.0 - 2.0	1	A-3	100	94	75	38	5	-	-	-	-	-	

Summary of Laboratory Test Results for Soil Classification														
Lena Road from North of 44th Avenue East to SR 64														
Manatee County, Florida														
Manatee County Project #6107560														
Tierra Project No.: 6511-22-127														
Boring Name	Sample Depth	Stratum	AASHTO			% Finer			Atte	rberg Li	mits	Organic	Moisture	
g	(feet)	•••••		#10	#40	#60	#100	#200	LL	PL	PI	Content	Content	
HA-FPC-13	2.5 - 3.5	1	A-3	100	90	68	35	6	-	-	-	2	23	
SH-249	0.5 - 1.5	1	A-3	-	-	-	-	6	-	-	-	-	-	
PBA-9	4.5 - 5.0	1	A-3	-	-	-	-	6	NP	NP	NP	-	22	
HA-FPC-7	1.5 - 2.0	1	A-3	-	-	-	-	6	-	-	-	3	24	
HA-FPC-3	1.5 - 2.0	1	A-3	-	-	-	-	6	-	-	-	-	-	
SH-222	2.5 - 3.0	1	A-3	-	-	-	-	6	-	-	-	3	22	
SH-279	0.5 - 1.5	1	A-3	I	-	-	-	6	-	-	-	-	-	
HA-FPC-12	2.0 - 3.0	1	A-3	-	-	-	-	6	-	-	-	4	27	
HA-FPC-6	1.5 - 2.0	1	A-3	100	91	69	33	6	-	-	-	-	-	
SH-222	1.5 - 2.0	1	A-3	100	93	73	39	6	-	-	-	-	-	
LBR-5	1.0 - 2.0	1	A-3	100	94	76	41	6	-	-	-	-	-	
SH-234	0.5 - 1.0	1	A-3	-	-	-	-	6	-	-	-	2	21	
SH-309	1.0 - 2.0	1	A-3	-	-	-	-	6	-	-	-	-	-	
HA-FPC-9	2.0 - 2.5	1	A-3	-	-	-	-	7	-	-	-	-	-	
B-258L	2.0 - 4.0	1	A-3	-	-	-	-	7	-	-	-	3	25	
LBR-3	1.0 - 2.0	1	A-3	100	92	74	36	7	-	-	-	-	-	
FPC-9	4.0 - 6.0	1	A-3	-	-	-	-	7	-	-	-	-	-	
LBR-4	1.0 - 2.0	1	A-3	100	90	71	36	7	-	-	-	-	-	
B-288R	13.5 - 15.0	1	A-3	-	-	-	-	7	NP	NP	NP	-	21	
AB-299R	2.5 - 5.0	1	A-3	-	-	-	-	9	-	-	-	-	19	
SH-ROUNDABOUT-2	3.0 - 3.5	1	A-3	-	-	-	-	9	-	-	-	2	20	
LBR-POND-2	1.0 - 2.0	1	A-3	100	88	65	29	9	-	-	-	-	-	
SH-219	4.5 - 5.0	1	A-3	-	-	-	-	9	-	-	-	-	-	
PBA-8	4.0 - 4.5	1	A-3	100	90	71	38	10	-	-	-	-	-	
HA-FPC-10	2.5 - 3.5	2	A-2-4	-	-	-	-	11	-	-	-	-	-	
PBA-4	0.0 - 0.5	2	A-2-4	-	-	-	-	12	-	-	-	4	24	
SH-232	0.0 - 0.5	2	A-2-4	-	-	-	-	12	-	-	-	4	27	

Summary of Laboratory Test Results for Soil Classification														
Lena Road from North of 44th Avenue East to SR 64														
Manatee County, Florida														
Manatee County Project #6107560														
Tierra Project No.: 6511-22-127														
Boring Name	Sample Depth	Stratum	AASHTO			% Finer			Atte	rberg Li	mits	Organic	Moisture	
Bornig Ramo	(feet)	otratam	///////////////////////////////////////	#10	#40	#60	#100	#200	LL	PL	PI	Content	Content	
LBR-2	1.0 - 2.0	2	A-2-4	100	95	78	42	12	-	-	-	-	-	
PBA-9	3.5 - 4.0	2	A-2-4	100	92	74	40	13	-	-	-	-	-	
HA-FPC-2	0.0 - 1.0	2	A-2-4	-	-	-	-	13	-	-	-	-	-	
AB-303R	3.0 - 5.0	2	A-2-4	-	-	-	-	13	NP	NP	NP	-	20	
FPC-7	2.0 - 4.0	2	A-2-4	-	-	-	-	14	-	-	-	-	-	
B-228R	13.5 - 15.0	2	A-2-4	-	-	-	-	14	-	-	-	-	-	
PBA-8	3.0 - 3.5	2	A-2-4	-	-	-	-	14	NP	NP	NP	-	20	
PBA-7	2.5 - 3.0	3	A-2-4	-	-	-	-	15	NP	NP	NP	-	19	
SH-ROUNDABOUT-2	3.5 - 4.0	3	A-2-4	-	-	-	-	15	NP	NP	NP	-	12	
HA-FPC-11	2.5 - 3.0	3	A-2-4	-	-	-	-	15	21	14	7	-	19	
SH-232	4.5 - 5.0	3	A-2-4	-	-	-	-	16	NP	NP	NP	-	22	
AB-305L	3.0 - 4.0	3	A-2-4	-	-	-	-	16	-	-	-	-	20	
PBA-9	3.0 - 3.5	3	A-2-4	-	-	-	-	16	NP	NP	NP	-	20	
AB-302R	2.0 - 2.5	3	A-2-4	-	-	-	-	17	NP	NP	NP	-	22	
SH-228	1.5 - 2.0	3	A-2-4	-	-	-	-	17	NP	NP	NP	-	22	
PBA-1	1.5 - 2.0	3	A-2-4	-	-	-	-	17	-	-	-	-	-	
SH-297	2.5 - 3.0	3	A-2-4	-	-	-	-	18	25	15	10	-	21	
B-233R	0.0 - 1.0	3	A-2-4	-	-	-	-	18	-	-	-	-	-	
HA-FPC-1	0.0 - 3.0	3	A-2-4	-	-	-	-	18	-	-	-	-	-	
SH-303	3.0 - 3.5	3	A-2-4	-	-	-	-	19	26	16	10	-	27	
PBA-1	0.0 - 0.5	3	A-2-4	-	-	-	-	20	-	-	-	-	-	
PBS-5	18.5 - 20.0	3	A-2-4	-	-	-	-	21	NP	NP	NP	-	27	
B-228L	0.0 - 2.0	3	A-2-4	-	-	-	-	22	NP	NP	NP	-	21	
B-298R	4.0 - 6.0	3	A-2-4	-	-	-	-	28	-	-	-	-	-	
LBR-POND-1	1.0 - 2.0	3	A-2-4	91	83	69	45	28	-	-	-	-	-	
AB-238R	0.5 - 2.5	3	A-2-4	-	-	-	-	29	NP	NP	NP	-	20	
PBS-6	23.5 - 25.0	3	A-2-4	-	-	-	-	35	NP	NP	NP	-	20	

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Manatee County, Florida														
Manatee County Project #6107560														
Tierra Project No.: 6511-22-127														
Boring Name	Sample Depth	Stratum	AASHTO			% Finer			Atte	rberg Li	mits	Organic	Moisture	
Bornig Namo	(feet)	otratam	///////////////////////////////////////	#10	#40	#60	#100	#200	LL	PL	PI	Content	Content	
PBS-9	23.5 - 25.0	4	A-2-6	-	-	-	-	21	37	25	12	-	41	
B-300L	2.0 - 4.0	4	A-2-6	-	-	-	-	22	28	16	12	-	25	
SH-234	2.5 - 3.0	4	A-2-6	-	-	-	-	26	28	9	19	-	32	
PBS-8	23.5 - 25.0	4	A-2-6	-	-	-	-	30	38	26	12	-	41	
B-219L	8.0 - 10.0	4	A-4	-	-	-	-	37	-	-	-	-	-	
B-243R	13.5 - 15.0	4	A-4	-	-	-	-	45	NP	NP	NP	-	27	
B-229R	33.5 - 35.0	4	A-4	100	97	89	70	51	-	-	-	-	-	
PBS-8	8.0 - 10.0	4	A-4	-	-	-	-	56	NP	NP	NP	-	20	
PBS-5	13.5 - 15.0	4	A-4	-	-	-	-	73	NP	NP	NP	-	28	
B-236L	8.0 - 10.0	4	A-6	-	-	-	-	45	27	16	11	-	23	
C-300L	13.5 - 15.0	4	A-6	-	-	-	-	76	39	26	13	-	26	
B-308R	13.5 - 15.0	4	A-6	-	-	-	-	78	39	23	16	-	46	
HA-FPC-2	1.0 - 2.0	5	A-8	-	-	-	-	10	-	-	-	12	42	
AB-234L	2.0 - 3.5	5	A-8	-	-	-	-	10	-	-	-	8	41	
AB-238R	3.0 - 4.5	5	A-8	-	-	-	-	10	-	-	-	12	52	
SH-237	1.5 - 2.0	5	A-8	-	-	-	-	13	-	-	-	12	52	
SH-237	1.0 - 1.5	5	A-8	-	-	-	-	14	-	-	-	11	47	
AB-233L	0.0 - 1.0	5	A-8	-	-	-	-	17	-	-	-	9	51	
AB-238R	2.5 - 3.0	5	A-8	-	-	-	-	18	-	-	-	28	133	
B-239R	0.0 - 0.5	5	A-8	-	-	-	-	29	-	-	-	14	57	
SH-228	2.0 - 2.5	5	A-8	-	-	-	-	32	-	-	-	19	89	
AB-236R	0.0 - 0.5	5	A-8	-	-	-	-	19	-	-	-	13	119	
B-228R	38.5 - 40.0	6	A-7-5	-	-	-	-	91	73	38	35	-	74	
B-227L	33.5 - 35.0	6	A-7-5	-	-	-	-	99	101	55	46	-	82	
PBS-8	13.5 - 15.0	6	A-7-6	-	-	-	-	44	49	27	22	-	59	
B-228R	2.0 - 4.0	6	A-7-6	-	-	-	-	53	47	21	26	-	46	
PBS-9	18.5 - 20.0	6	A-7-6	-	-	-	-	73	57	29	28	-	49	

	Summary of Laboratory Test Results for Environmental Classification Lena Road from North of 44th Avenue East to SR 64 Manatee County, Florida Manatee County Project #6107560 Tierra Project No.: 6511-22-127												
Boring Name	BoringDepth (feet)BratumPH (FM 5-550)Resistivity (ohm-cm)Chlorides (ppm)Sulfates (ppm)Environmental Classification* (Soil)												
Name	(1001)		(1 11 0 000)	(FM 5-551)	(FM 5-552)	(FM 5-553)	Steel	Concrete					
SH-219	0.0 - 1.0	1	7.4	4,200	90	<5	Moderately Aggressive	Slightly Aggressive					
SH-234	0.0 - 0.5	1	7.8	2,300	90	270	Moderately Aggressive	Moderately Aggressive					
SH-249	0.5 - 1.5	1	7.4	9,400	45	<5	Slightly Aggressive	Slightly Aggressive					
SH-264	0.5 - 1.5	1	7.8	20,000	30	<5	Slightly Aggressive	Slightly Aggressive					
SH-279	0.5 - 1.5	1	7.8	8,200	30	<5	Slightly Aggressive	Slightly Aggressive					
SH-294	0.5 - 1.5	1	8.1	12,000	30	<5	Slightly Aggressive	Slightly Aggressive					
SH-309 1.0 - 2.0 1 8.1 8,600 30 15 Slightly Aggressive Slightly Aggressive													
* Based on	Based on the current FDOT Structures Design Guidelines												

Hydraulic Conductivity Test Results Lena Road from North of 44th Avenue East to SR 64 Manatee County, Florida Manatee County Project #6107560 Tierra Project No: 6511-22-127										
Boring Number	Boring Location <sup>(1)</sup>				Test	Test Depth	Stratum	Percentage	Vertical Unsaturated Hydraulic Conductivity.	Horizontal Saturated Hydraulic Conductivity.
	Station Off (feet) (fe	Offset (feet)	Easting (feet)	Northing (feet)	Designation	Surface (feet)	Tested	#200 Sieve	(feet/day) <sup>(2)</sup>	k <sub>H</sub> (feet/day) <sup>(2)</sup>
DRI-1	296+52	37' RT.	508126	1144787	DRIT	1	1	N/A	11.0	11.0
<ol> <li>Boring location was determined using the GPS coordinates obtained in the field and therefore should be considered approximate. The coordinates are referenced to the FL State Plane West Coordinate System.</li> <li>The hydraulic conductivity values presented are for the soil stratum indicated in the table and are not factored. The design engineer should apply an appropriate factor of safety.</li> <li>N/A: Not Available</li> </ol>										

Design LBR Calculation - 2% of Optimum Method Lena Road from North of 44th Avenue East to SR 64 Manatee County, Florida Manatee County Project #6107560 Tierra Project No.: 6511-22-127							
Test No.	Bulk Sample	Maximum	LBR at Moisture Contents (of Optimum LBR):				
	Location	LBR	- 2%	+ 2%			
LBR #1	LBR-1	49	41	33			
LBR #2	LBR-2	66	29	47			
LBR #3	LBR-3	45	36	23			
LBR #4	LBR-4	32	29	24			
LBR #5	LBR-5	31	24	14			
LBR #6	LBR-6	33	25	15			
LBR #7	LBR-7	25	18	8			
LBR #8	LBR-8	57	47	35			
LBR #9	LBR-9	67	47	34			
Mean Ll	BR Value	45	33	26			
Design LBR = 29							
Design M <sub>R</sub> (Resilent Modulus) <sup>(1)</sup> =  9,750 psi							
<sup>(1)</sup> Based on FDOT Flexible Pavement Manual for conversion of LBR to M <sub>R.</sub>							

r

Design LBR Calculation - 90% Method Lena Road from North of 44th Avenue East to SR 64 Manatee County, Florida Manatee County Project #6107560 Tierra Project No.: 6511-22-127							
Test No.	Bulk Sample Location	Maximum LBR	Rank	Percent of Samples with equal or greater value			
LBR #7	LBR-7	25	1	100			
LBR #5	LBR-5	31	2	89			
LBR #4	LBR-4	32	3	78			
LBR #6	LBR-6	33	4	67			
LBR #3	LBR-3	45	5	56			
LBR #1	LBR-1	49	6	44			
LBR #8	LBR-8	57	7	33			
LBR #2	LBR-2	66	8	22			
LBR #9	LBR-9	67	9	11			
Design LBR = 30 Design M <sub>R</sub> (Resilent Modulus) <sup>(1)</sup> =  10,000 psi							
<sup>(1)</sup> Based on FDOT Flexible Pavement Manual for conversion of LBR to M <sub>R.</sub>							
























sted For: Kir	mley-Horn a	nd Asso	ociates, li	nc.					Project	:	Lena Roa Manatee	d County,	, Florida					
Date: 5/2	ate: 5/2/2023										Project No. 6511-22-127 Report No. LBR-POND-4 296+54, 63' RT.							
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ested For:	Kimley-Horn and Associates, Inc.									Project: Lena Road Manatee County, Florida								
Date:	e: 5/2/2023										Project No. 6511-22-127 Report No. LBR-POND-5 301+14, 250' RT.							
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# **APPENDIX E**

Pavement Data and Condition Sheet

#### Pavement Data and Condition Sheet Lena Road from North of 44th Avenue East to SR 64 Manatee County, Florida Manatee County Project #6107560 Tierra Project No.: 6511-22-127

Approximate Core Location <sup>(1)</sup> (FL West NAD 83) Asphalt Layer Base for Paved Roadway Subgrade F Rut Depth Groundwater	ments
Lane Thickness out of the Crack Depth Pavement Rut Depth Groundwater	ments
Core No. Designation Type <sup>(2)</sup> Total Asphalt Total Asphalt	
Lasting (feet) Northing (feet) Top Layer 2nd Layer 3rd Layer Top Layer 2nd Layer 3rd Layer Core Length (inches) Type Inickness (inches) Type Depth <sup>1/7</sup> (feet)	
C-1     507883     1139354     Northbound     1.0     3.0     1.4     S-3     SP-9.5     S-3     5.4     Limerock     8.7     A-3     1.2 to 4.5 <sup>(6)</sup> 1.9% Outside     GNE     Boring terminated due to the second	refusal conditions
C-2   507890   1140057   Northbound   0.8   1.6   1.4   S-3   SP-12.5   S-1   3.8   Soil Cement   8.2   A-3   1 to 5 <sup>(3)</sup> F <sup>(6)</sup> 3.0% Outside   4.7	
C-3   507955   1140513   Southbound   4.5    S-1    4.5   Sand w/Shell   5.5   A-3   0.8 to 5 <sup>(3)</sup> G   0.1   2.6% Outside   GNE	
C-4 508107 1140898 Northbound 1.0 1.4 1.9 S-3 SP-9.5 S-1 4.3 Soil Cement 6.7 A-3 0.9 to 5 <sup>(3)</sup> G <sup>(6)</sup> 2.8% Outside 4.5	
C-5 508108 1141496 Southbound 3.0 1.4 SP-12.5 S-3 4.4 ABC-3 SAHM 2.1 A-3 0.9 to 5 3.0 F <sup>(6)</sup> 1.6% Outside 4.6	
C-6     508133     1142060     Northbound     2.3     1.2      SP-12.5     S-3      3.5     ABC-3 SAHM     1.8 4.7     A-3     0.8 to 5     2.0     P     0.3     0.7% Outside     GNE     Sloughing	
C-7   508128   1142369   Southbound   2.2   0.9    S-1   S-3    3.1   ABC-3 SAHM   2.9 4.0   A-3   1.3 to 5  (3)   F   0.1   1.9% Outside   GNE	
C-8   507993   1143211   Northbound   1.6   2.2    SP-12.5   S-3    3.8   ABC-3   2.3   A-3   1.5 to 5   1.8   P   0.1   0.3% Outside   GNE	
C-9 507972 1143808 Southbound 3.0 1.3 S-1 S-3 4.3 ABC-3 SAHM 1.2 A-3 0.9 to 5 3.0 P 0.1 3.3% Outside GNE	
C-10     508004     1144248     Northbound     2.5     1.8      SP-12.5     S-3      4.3     ABC-3 SAHM Sand w/Shell     3.3 5.3     A-3     1.3 to 5     3.3     P     0.1     0.5% Outside     4.4	
C-11   508013   1144753   Southbound   3.4   1.5    S-1   S-3    4.9   ABC-3 SAHM   4.5 1.8   A-3 A-2-4   0.9 to 4.5 4.5 to 5   4.9 <sup>(4)</sup> P   0.1   2.3% Outside   GNE	
C-12     508063     1145525     Northbound     1.5     1.5      SP-9.5     S-3      3.0     ABC-3 SAHM Sand w/Shell     2.0 5.0 6.0     A-3 A-2-4     1.3 to 4.5 4.5 to 5 <sup>(3)</sup> G <sup>(6)</sup> 3.1% Outside     3.8     New Pavement	
Notes:	
<sup>(1)</sup> Pavement Core locations were estimated utilizing GPS coordinates referenced to the Florida State Plane West coordinate system obtained by Tierra, Inc. in the field and should be considered approximate.	

assified in descending order from the top ot the tual pavement layer may be a local mix. Pavement layers are sample

<sup>(3)</sup> No cracks were observed within the pavement cores at these locations.

<sup>(4)</sup> Full depth cracking observed within the pavement core at these locations.

<sup>(5)</sup> Pavement condition based on visual observation; Good, Fair, or Poor.

<sup>(6)</sup> No visible ruts were observed at the pavement core locations.

<sup>(7)</sup> Depth measured from pavement surface. SAHM: Sand Asphalt Hot Mix



Photograph 1. Pavement Core No. C-1 Field

Photograph 2. Pavement Core No. C-1 Top View

Photograph 3. Pavement Core No. C-1 Side View



Photograph 4. Pavement Core No. C-2 Field

Photograph 5. Pavement Core No. C-2 Top View

Photograph 6. Pavement Core No. C-2 Side View



Photograph 7. Pavement Core No. C-3 Field

Photograph 8. Pavement Core No. C-3 Top View

Photograph 9. Pavement Core No. C-3 Side View



Photograph 10. Pavement Core No. C-4 Field

Photograph 11. Pavement Core No. C-4 Top View

Photograph 12. Pavement Core No. C-4 Side View



Photograph 13. Pavement Core No. C-5 Field

Photograph 14. Pavement Core No. C-5 Top View

Photograph 15. Pavement Core No. C-5 Side View



Photograph 16. Pavement Core No. C-6 Field

Photograph 17. Pavement Core No. C-6 Top View

Photograph 18. Pavement Core No. C-6 Side View



Photograph 19. Pavement Core No. C-7 Field

Photograph 20. Pavement Core No. C-7 Top View

Photograph 21. Pavement Core No. C-7 Side View



Photograph 22. Pavement Core No. C-8 Field

Photograph 23. Pavement Core No. C-8 Top View

Photograph 24. Pavement Core No. C-8 Side View



Photograph 25. Pavement Core No. C-9 Field

Photograph 26. Pavement Core No. C-9 Top View

Photograph 27. Pavement Core No. C-9 Side View



Photograph 28. Pavement Core No. C-10 Field

Photograph 29. Pavement Core No. C-10 Top View

Photograph 30. Pavement Core No. C-10 Side View



Photograph 31. Pavement Core No. C-11 Field

Photograph 32. Pavement Core No. C-11 Top View

Photograph 33. Pavement Core No. C-11 Side View



Photograph 34. Pavement Core No. C-12 Field

Photograph 35. Pavement Core No. C-12 Top View

Photograph 36. Pavement Core No. C-12 Side View

# **APPENDIX F**

Test Pit Sample Photographs



**Historical Photograph** 



Test Pit TP-11



Test Pit TP-11



Test Pit TP-21



Test Pit TP-21



Test Pit TP-21



Test Pit TP-25



Test Pit TP-25



Test Pit TP-25



Test Pit TP-15
Test Pit Sample Photographs Lena Road – Parcel 103 Manatee County, Florida Manatee County Project #6107560 Tierra Project No. 6511-22-127



Test Pit TP-15