# REPORT OF THE GEOTECHNICAL INVESTIGATION

LIFT STATION 9-A REHABILITATION AND FORCE MAIN MANATEE COUNTY, FLORIDA

October 29, 2021

INCORPORATED

Jacobs Engineering Group, Inc 4350 W. Cypress Street, Suite 600 Tampa, FL 33607

IGGERS

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Attn: Ms. Michelle Collins, P.E.

RE: Report of the Geotechnical Investigation Lift Station 9-A Rehabilitation and New Force Main Manatee County, Florida Our File: DES 218808

Dear Ms. Collins:

DRIGGERS ENGINEERING SERVICES, INC. has completed the requested subsurface investigation program for the subject project. Presented herein are the results of our field and laboratory tests, together with a discussion of our findings and recommendations.

NGINEERING

SERVICES

Geotechnical Engineering & Construction Materials Testing

### SUBSURFACE INVESTIGATION PROGRAM

Plate I of the report illustrations identifies the respective positioning of a series of test borings that have been completed to investigate subsurface conditions. The program consisted of seven (7) Standard Penetration Test (SPT) borings and nine (9) classification or hand auger borings with accompanying hand cone penetrometer test. The hand auger borings were advanced to depths of 10 feet below existing grade within the planned force main alignment while the SPT borings penetrated to a depth of 10 to 40 feet below existing grade within the planned lift station location and selected pipeline alignment locations. The locations depicted on Plate I are approximate. Logs of the test borings are presented in the report attachments reflecting visual together with estimated Unified Soil classification. Soils were logged in the field by our geotechnician, with representative soil samples sealed in glass jars and returned to the laboratory for further examination by the project engineer and development of boring logs. A log of each hand auger boring is attached.

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#### LABORATORY INVESTIGATION

A limited laboratory testing program was undertaken to aid in characterizing the engineering properties of the subsurface soils. Our laboratory tests included grainsize analyses and organic content tests. The results of our laboratory tests are included in the report attachments.

#### **GENERALIZED SUBSURFACE CONDITIONS**

<u>SOIL AND GROUNDWATER CONDITIONS</u> - The borings have identified predominantly fine sands with variable silt and clay fines to a depth of 27 feet below existing grade. The soils within these depths varied from very loose to medium dense in relative density and generally have a Unified Classification of SP, SP-SM, SM and SC. Below 27 feet, the lift station boring encountered the limestone formation to the termination depth of the boring at 40 feet below grade.

It should be noted that boring SPT-3 identified a highly organic soil at 3.4 to 4.4 feet below existing grade. Furthermore, borings SPT-7 and HA-6 encountered a slightly organic fine sand with roots and a slightly silty fine sand with roots at 4.6 to 6.0 and 3.0 to 4.0 feet below existing grade, respectively.

<u>GROUNDWATER CONDITIONS</u> - Groundwater was recorded in early October of 2021 at 3.0 to 4.8 feet below grade at the test locations. These variations in the groundwater depths are most likely attributed to surface elevation changes along the proposed alignment. Considering the water levels were recorded near the end of the wet season, we would expect the groundwater levels to rise from their current levels at the peak of a typical wet season.

#### EVALUATION AND GEOTECHNICAL RECOMMENDATIONS

<u>**PROJECT DESCRIPTION**</u> – It is our understanding that the project consists of the installation of a nominal 12-inch diameter PVC water line using open-cut direct burial. It is also our understanding that open-cut installation sections of the water main will be embedded at about 4.0 feet below existing grade.

**<u>DIRECT EMBEDMENT</u>** – In accordance with our discussions, the pipeline will be embedded with a minimum of 3 feet of cover. Thus, one would anticipate that the pipe embedment would typically be in the range of perhaps 4 feet below existing grade.

The soils typically encountered within and just below this anticipated depth of embedment would predominately consist of fine sands with variable silt content, which should be suitable for pipe support as well as provide suitable material for backfill placement. However, as previously discussed, unsuitable highly organic soils and soils with varying concentrations were identified within a few boring locations that would not be considered suitable. Careful geotechnical inspection is recommended during construction to help identify any unsuitable soils or materials that may warrant removal and replacement. Where evidenced, these materials should be removed and replaced with suitable bedding material as directed by the project engineer. Furthermore, such zones with excessive silt, clay and organic content may not meet project specification requirements with respect to their usage as suitable backfill. Where directed, these soils should be removed and disposed of as directed by project specification requirements. It may be prudent to also allow for offset borings at boring locations SPT-3, SPT-7 and HA-6 to aid in determining the vertical and lateral extent of evidenced unsuitable soils prior to construction.

In accordance with project specifications, it is our understanding that compaction should achieve a minimum density of no less than 98% of the Modified Proctor, maximum dry density established in ASTM D-1557 under roadways while a minimum density of no less than 95% of the Modified Proctor, maximum dry density will be required for all other compaction applications including the pipe placement and pipe backfill. Identified suitable soil conditions would be considered suitable to achieve the recommended compaction requirements.

**EXCAVATION STABILITY AND MANAGEMENT OF GROUNDWATER** – It is anticipated that the direct embedded pipeline section of the project will be constructed in an open excavation or perhaps utilizing trench box construction. With all the excavation activities and considering the existing shallow groundwater level at various locations along the alignment, it is anticipated that control and management of groundwater will be important to maintain excavation stability, allow appropriate placement of piping to line and grade and to permit proper backfill placement and compaction. It is recommended that groundwater levels be maintained no less than one (1) foot below the maximum depth of excavation or deeper as needed, dependent upon the ways and means of construction. It is envisioned that the de-watering, where needed, will be accomplished utilizing an appropriately designed well-point system. Open excavation areas should be appropriately sloped in accordance with applicable OSHA Trench Safety requirements. The excavation contractor should certainly exercise due care with respect to identification and protection of any existing structures or utilities that are within the area of influence of his work activities.

<u>**PROPOSED LIFT STATION</u>** - It is our understanding, based on preliminary information provided by your office that the proposed pump station will include a single pump can or wet well with the bottom of the pump can foundation at about 27 feet below existing grade.</u>

A variably cemented silty fine sand was encountered at a depth of about 23 feet below grade and continued to a depth of about 27 feet where the limestone formation was penetrated to the completion depth of the borings at 40 feet below grade. The bottom of the excavation should be carefully inspected and probed by a representative of the project geotechnical engineer and undercut at least 6 inches, replaced with a utilizing crushed concrete, crushed granite or hard durable crushed limerock having a grading corresponding to an FDOT No. 57 over an appropriate geotextile fabric to avoid disturbance and remolding of subgrade soils due to rain, foot traffic, etc. Following proper subgrade preparation as recommended herein, we would anticipate settlements of the wet well of less than 1 inch. We would expect these settlements would occur virtually coincident with the replacement of backfill soils and pump can filling. Further, we would anticipate relatively uniform settlement provided that appropriate plumbness is maintained during backfilling operations and extraction of any sheeting utilized.

<u>SOIL STRENGTH PARAMETERS</u> - It is our understanding that there will be the need for various earth retention systems to facilitate construction as well as other below grade structures. Accordingly, the following geotechnical parameters are considered preliminary for use in the analyses of the various structures.

Soil Consistency	Total Unit Weight (pcf)	Buoyant Unit Weight (pcf)	Angle of Internal Friction, φ	Active Earth Pressure Coefficient (Ka)	At-Rest Earth Pressure Coefficient (Ko)	Passive Earth Pressure Coefficient (Kp)
<u>Very loose</u> fine sands and silty sands	115	55	28	0.36	0.53	2.7
<u>Loose</u> fine sands and silty sands	120	60	30	0.33	0.5	3.0
<u>Medium dense</u> fine sands and silty sands	120	60	32	0.30	0.47	3.33

Note: Properly compacted sands and non-plastic slightly silty and silty sands would likely possess a medium dense relative density for use in analyses.

#### **LIMITATIONS**

The geotechnical investigation herein was performed to obtain subsurface information to help facilitate the design of the planned utilities. Our geotechnical investigation may not have included the gathering of all information that may be desired by the respective contractor in the preparation of his bid proposal, or in the development of his ways and means of construction. Each contractor is encouraged to conduct such additional investigative effort or testing that he may deem appropriate to prepare his bid proposal and construction plan.

DRIGGERS ENGINEERING SERVICES, INC. appreciates the opportunity to serve you and we trust that if you have any questions concerning our report, you will not hesitate to contact this office at your convenience.

Respectfully Submitted, DRIGGERS ENGINEERING SERVICES, INC.

A. Driggers, P.E. Wice President

FL Registration No. 70598



JAD JAD-REP\218808 Copies submitted: (1) Email

# APPENDIX

# PLATE I - BORING LOCATION PLAN

# STANDARD PENETRATION TEST BORING LOGS

# HAND AUGER BORING / HAND CONE SOUNDING LOGS

#### SUMMARY OF LABORATORY TEST RESULTS

GRAINSIZE ANALYSES

### **METHOD OF TESTING**

Driggers Engineering Services Incorporated

# PLATE I - BORING LOCATION PLAN























STANDARD PENETRATION TEST BORING LOGS

DR	IGG	ERS ENGINEERING SER'	VICES	INCORPORATED
		DES 218808 BORING NO. SF Station 9A Rehabilitation & New Force Main, Manate		da
Loca	tion S	See Plate I-A		an N.P.
Com De	pletio epth	Depth To           40.0'         Date         10/1/21         Water         3.1'	_ Time	<b>Date</b> 10/1/21
<b>DEPTH, FT</b>	SYMBOL	SOIL DESCRIPTION	BLOWS ON SAMPLER PER 6" OR PEN. STR.	STANDARD PENETRATION TEST BLOWS/FT. ON 2" O.D. SAMPLER-140 LB. HAMMER, 30" DROP
0		SURF. EL:	6" S	10 20 40 60 80
		Dark brown highly organic, silty Fine SAND (with roots (Pt) (A-8)		
		Brown Fine SAND (SP) (A-3)		
		Light brown and brown Fine SAND (SP) (A-3) Dark brown slightly organic, slightly silty	_	
- 5 -		Fine SAND (SP-SM) (A-3)		
		Very loose to dense light brown Fine SAND		
		(SP) (A-3)	1/1/2	•
			2/1/3	
- 10 -				
			6/6/11	
			8/7/5	
			0///3	
- 15 -				
15			9/21/28	
		Modium donao graviah braun Eina SAND	_	
		Medium dense grayish-brown Fine SAND (SP) (A-3)		
- 20 -			44/44/40	
			14/14/13	
		Very dense grayish-green variably cemented,		
- 25 -		silty Fine SAND (SM) (A-2-4)		
25			2/8/50*	* 0.2' Penetration

Remarks Borehole Grouted			
	Cas	ing Ler	ngth

\* 0.1' Penetration-

50\*

Cream colored LIMESTONE

30



			DES 21		-	BORING								
			e Plate		bilitation & N	lew Force Main	n, Manatee C	Forema			N.P.			
Comp		n	Flate	I-A		Depth To					11.1 .		-	
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<b>DEPTH, FT</b>	SYMBOL	SAMPLES	SURF		SOIL DES	CRIPTION		BLOWS ON SAMPLER PER 6" OR PEN. STR.	BL S H/	STA ENETR/ OWS/F SAMPLI AMMER	ATIO T. ON ER-14	N TE I 2'' ( 40 LI '' DR	D.D 3. OP	).
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			(SP) (A-3)	_				+	$\left  \right $	Ħ
			Light brown Fine SAND (SP) (A-3)						Ħ	Ħ
			Tan Fine SAND (SP) (A-3)	1 [						
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	Completion Depth       16.5'       Date       10/4/21       Water       4.0'       Time       Date         Li       Hi       SOIL DESCRIPTION       NO SURF.EL:       NO SURF.EL:       NO SURF.EL:       NO SURF.EL:       10 2         0       Esses       Brown Fine SAND with roots and gravel       10       2       10       2         5       Interview       Tan Fine SAND (SP) (A-3)       Interview       Interview       Interview					-	$\vdash$	╂		
	Project Lift Station 9A Rehabilitation & New Force Main, Manatee County, Florida         Location See Plate I-A		┢	$\vdash$	Ħ					
- 10 -		1		7/8/4					Ħ	Ħ
		4								Π
		Λ	Medium dense light brown Fine SAND (SP) (A-3)	8/9/14		<u></u>				Ц
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- 10 -	().:	1	Medium dense						$\overline{}$	-	H	+	
- 15 -			slightly silty Fin		<u>-SM) (A-3)</u>		5/9/11						
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-		_	BORING NO. SPT							
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				8/18/24						
- 10 -				15/18/14						
- 15 -										
- 30 -										
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Rem	narks	Bo	rehole Grouted	Cas	ing Leng	th		<u>+</u>		<u>+++</u>
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DRIGGERS	ENGINEERING	SERVICES	INCORPORATED

		_	BORING NO. SPT							
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Com	oletio	n	Depth To							—
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DEPTH, FT		SAMPLES	SOIL DESCRIPTION	BLOWS ON SAMPLER PER 6" OR PEN. STR.	BLC S/	STAN NETRAT DWS/FT. AMPLEF MMER, D 20	TION 2 ON 2 R-140 30'' E	TES 2" C LB DRC	).D. 8. 0P	80
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- 10 -			With roots (SP-SM) (A-3)         Dark brown slightly silty Fine SAND (SP-SM) (A-3)         Grayish-brown Fine SAND (SP) (A-3)         Loose light brown slightly silty Fine SAND (SP-SM) (A-3)         Medium dense light brown Fine SAND (SP) (A-3)         Medium dense brown slightly silty Fine SAND (SP-SM) (A-3)	6/5/4						
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- 30 -							_	$\left  + \right $	+	
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DRIGGERS         ENGINEERING         SERVICES         INCORPORATE           Project Lift Station 9A Rehabilitation & New Force Main, Manatee County, Florida												AL AL ALLAND
Project         Lift Station 9A Rehabilitation & New Force Main, Manatee County, Florida           Location         See Plate I-J         Foreman         N.P.           Completion         11.5'         Date         10/5/21         Water         4.5'         Time         Date         10/5/21           Lit         To         SOIL DESCRIPTION         StandARD         StandARD         PENETRATION TEST         BLOWS/FT. ON 2'' OL'S AMPLER: 140 LB.           SURF. EL:         SOIL DESCRIPTION         StandBack         HAMMER, 30'' DROP         10         20         40         60           SURF. EL:         Grayish-brown Fine SAND with roots and shell         StandBack         10         20         40         60           Light brown Fine SAND (SP) (A-3)         Light brown Fine SAND (SP) (A-3)         5/5/3         40	DRI	GG	E	RS ENGINEERING SERVICI	ES II	NCC	DRP	OR	A	Т	EI	
Location         See Plate I-J         Foreman         N.P.           Completion         Date         10/5/21         Water         4.5'         Time         Date         10/5/21           Ling         SOIL DESCRIPTION         Solution         Solution <t< td=""><td></td><td></td><td>_</td><td></td><td></td><td>_</td><td></td><td></td><td></td><td></td><td></td><td>_</td></t<>			_			_						_
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DF		ERS ENGINEERING SERV	/ICES	INCORPORATED
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<b>DEPTH</b> , FT		SOIL DESCRIPTION	BLOWS ON SAMPLER PER 6" OR PEN. STR.	STANDARD PENETRATION TEST BLOWS/FT. ON 2" O.D. SAMPLER-140 LB. HAMMER, 30" DROP
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Remarks Borehole Grouted

**Casing Length** 

HAND AUGER BORING / HAND CONE SOUNDING LOGS



# DRIGGERS ENGINEERING SERVICES INCORPORATED

	HAND AUGER BORING		2000		~			
PROJE	CT: Lift Station 9A Rehabilitation & New Force Main	CLIENT:		:l.	acobs			
	Manatee County, Florida Project No.: DES 218808	WATER TA	ABLE:	3.1'		DATE:	0/1/21	
TECHN	ICIAN:	DATE:	40/4/5		COMPLI	ETION DEP	TH:	
LOCAT		TEST NUM	10/1/2 IBER:			6.0'		
	See Plate I-A	<del>                                      </del>		S	HAND CO			
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	<ul> <li>+ Denotes Penetration Resistance in excess of 50 TSF</li> </ul>	- 7 -						
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	HAND AUGER BORING/H		G LOG						
PROJECT: Lift Station 9A Rehabilitation & New Force Main		CLIENT: Jacobs							
	Manatee County, Florida Project No.: DES 218808	WATER TABLE: 4.0'	DATE: 10/4/21						
TECHNICIAN:		DATE:	COMPLETION DEPTH: 6.0'						
LOCATIO	N.P./K.A.	10/4/21 TEST NUMBER:							
	See Plate I-A		SPT-2 HAND CONE TIP						
ELEV. (FT)	DESCRIPTION	DEPTH OB (FT) X 0 10	RESISTANCE (TSF)						
	Brown Fine SAND with roots and gravel (SP) (A-3)								
	Light brown Fine SAND (SP) (A-3)								
-	Tan Fine SAND (SP) (A-3)	3	+						
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	<ul> <li>+ Denotes Penetration Resistance in excess of 50 TSF</li> </ul>	- 7 -							



PROJEC		IAND CONE SOUNDING LOG	
PROJEC	& New Force Main	Jacobs	
Manatee County, Florida		WATER TABLE: DATE 3.1'	: 10/4/21
Project No.: DES 218808		DATE: COMPLETION DE	EPTH:
LOCATIO	N.P./K.A.	10/4/21 6.0 TEST NUMBER:	)'
LUCATIO	See Plate I-C	SPT-3	
		L HAND CONE TIP	)
ELEV. (FT)	DESCRIPTION	DEPTH (FT)	
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	(SP) (A-3)		
	Light brown slightly silty Fine SAND		
	(SP-SM) (A-3)		
		2	
	Grayish-brown slightly silty Fine SAND		
	(SP-SM) (A-3)	Liji i j.d	
	Grayish-brown clayey Fine SAND		
	(SC) (A-2-6)		
	Dark gray highly organic,		
	silty Fine SAND with fibrous matter		
	(Pt) (A-8)		
		- 4 -	*+
	Light brown Fine SAND (SP) (A-3)		• +
		- 5	↓
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	LEGEND:		
	• + Denotes Penetration Resistance		
	in excess of 50 TSF		



	HAND AUGER BORING	HAND CONE SOUNDING LOG								
PROJE	CT: Lift Station 9A Rehabilitation & New Force Main	CLIENT: Jacobs								
	Manatee County, Florida	WATER TABLE:         DATE:           4.8'         10/4/21								
Project No.: DES 218808		DATE: COMPLETION DEPTH:								
LOCATI	N.P./K.A.	10/4/21 6.0' TEST NUMBER:								
	See Plate I-E	SPT-4 HAND CONE TIP								
ELEV. (FT)	DESCRIPTION	DEPTH C RESISTANCE (TSF)								
	Dark brown Fine SAND with roots (SP) (A-3)									
	Brown slightly silty Fine SAND (SP-SM) (A-3)	1         1           1         1								
	Dark brown slightly silty Fine SAND (SP-SM) (A-3)	2 100000 100000 100000 10000 10000 10000 10000 10000 10000 100								
	Brown Fine SAND (SP) (A-3)									
s.	<ul> <li>LEGEND:</li> <li>● + Denotes Penetration Resistance in excess of 50 TSF</li> </ul>									



	HAND AUGER BORING/	IAND CONE SOUNDING	G LOG							
PROJEC	CT: Lift Station 9A Rehabilitation	CLIENT: Jacobs								
	& New Force Main Manatee County, Florida Project No.: DES 218808	WATER TABLE: 3.0'	DATE: 10/4/21							
TECHNI		DATE: 10/4/21	COMPLETION DEPTH: 6.0'							
LOCATI	ON:	TEST NUMBER:	SPT-5							
	See Plate I-G		HAND CONE TIP							
ELEV. (FT)	DESCRIPTION	DEPTH OB (FT) X 0 10	RESISTANCE (TSF) 20 30 40 50 60 70							
	Dark grayish-brown slightly silty Fine SAND with roots (SP-SM) (A-3) Dark brown slightly silty Fine SAND (SP-SM) (A-3)	0 +)								
	Grayish-brown Fine SAND (SP) (A-3)	3								
	Light brown slightly silty Fine SAND (SP-SM) (A-3)		•+							
	LEGEND:	- 6								
	• + Denotes Penetration Resistance									
	in excess of 50 TSF	- 7 -								



	HAND AUGER BORING/	HAND CO	NE SO	UNDI	NG LC	)G				
PROJEC	T: Lift Station 9A Rehabilitation & New Force Main	CLIENT:				lacobe				
K New Force Main Manatee County, Florida Project No.: DES 218808 TECHNICIAN: N.P./K.A.		UATER TABLE: 4.5' DATE: 10/5/21					DATE: 10/5/21			
							COMPLETION DEPTH: 6.0'			
LOCATIO	DN: See Plate I-J	TEST NUMBER: SPT-6								
ELEV. (FT)	DESCRIPTION	DEPTH (FT)	SYMBOL				ID CON STANCE	E TIP E (TSF)		
	Grayish-brown Fine SAND	0	S S	0 1	02	<u>:0 3</u>	30 4	40 50	) 6	0 70
	with roots and shell (SP) (A-3)		₹.\$.\$						+	
	Light brown Fine SAND with shell (SP) (A-3)		▽ . ▽ . ▽							
		- 1 -	∨.∨ 7.⊽.⊽ ⊽.⊽.v					+	+	
	Brown Fine SAND (SP) (A-3)		₩ ₩ ¥						+	
		2 -			_		[			
	Light brown Fine SAND (SP) (A-3)									
					)					
		- 3 -			$\leq$					
									+	
		- 4 -							+	
		- 5 -			<b>\</b>					
		197								
-		- 6 -							+	
	LEGEND:									
	<ul> <li>+ Denotes Penetration Resistance in excess of 50 TSF</li> </ul>	- 7 -								



# DRIGGERS ENGINEERING SERVICES INCORPORATED

<b>DDO 150</b>	HAND AUGER BORING								
PROJEC	CT: Lift Station 9A Rehabilitation & New Force Main	CLIENT: Jacobs							
Manatee County, Florida		WATER	TABLE:	4.3'			DAT	E:	21
Project No.: DES 218808		DATE:		10/5/21 COMPLETION DEPTH:					
	N.P./K.A.		10/ UMBER:	5/21			6.	.0'	
LOOAIIC	See Plate I-L				SF	PT-7			
ELEV.		DEPTH	g	HAND CONE TIP RESISTANCE (TSF)					
(FT)	DESCRIPTION	(FT)	SYMBOL	0 10		30	40	50	60 70
	Grayish-brown Fine SAND with roots (SP) (A-3)	0	×						
	Brown Fine SAND (SP) (A-3)							• +	
		- 1							
	Grayish-brown Fine SAND (SP) (A-3)							+	
		- 2 -							
								ť	
		- 3 -							
	Gray Fine SAND (SP) (A-3)								
	Dark grayish-brown Fine SAND (SP) (A-3)	4							
	Dark brown slightly organic, silty Fine SAND with roots (SM) (A-2-4)	- 5 -							
			11111111111111111111111111111111111111	•					
·		6 -		┝╺┼			_	_	
	LEGEND:								
	• + Denotes Penetration Resistance								
	in excess of 50 TSF	- 7 -	1					_	


PROJEC		CLIENT:								
	& New Force Main Manatee County, Florida Project No.: DES 218808	WATER	TABLE:		1.2'	Jacobs	S	DATE	: 10/6/2	1
TECHNI	CIAN:	DATE:	10/	6/21		С	OMPL	ETION DE 6.2	EPTH:	
LOCATIO	K.A ON:	TEST NU	JMBER:	0/21				0.2		
	See Plate I-A					<u>HA-1</u> HA		ONE TIP		
ELEV. (FT)	DESCRIPTION	DEPTH (FT)	SYMBOL	0	10	RES	30	CE (TSF)	50	60 70
	Brown slightly silty Fine SAND (SP-SM) (A-3)	0	1:1:1:1 1:1:1:1 1:0::::::						+	
	Light brown Fine SAND with trace of rock fragments (SP) (A-3)								• +	
	Grayish-brown Fine SAND (SP) (A-3)								• +	
	Dark grayish-brown Fine SAND (SP) (A-3)	_ 2		•			+		T	
	Brown Fine SAND (SP) (A-3)								• + • +	
	Light brown slightly silty Fine SAND (SP-SM) (A-3)	4	11:1:1:1:1 11:1:1:1:1 11:1:1:1:1 11:1:1:1:1	1		$\leftarrow$			•+	
	Brown slightly silty Fine SAND (SP-SM) (A-3)		11.11. 11.11.1						<b>+ +</b>	
	Dark reddish-brown weakly cemented, slightly silty Fine SAND (SP-SM) (A-3)	- 6	1011). 1011). 1011)							
	<ul> <li>Could not penetrate below depth 6.2' due to cemented sands.</li> </ul>		-							
	Note: Borehole Grouted	- 8	-							
			-							
		- 10 ·								
			-							
		- 12 -			_		_			
	LEGEND:									
	<ul> <li>+ Denotes Penetration Resistance in excess of 50 TSF</li> </ul>		-							
		- 14 -	-	<u> </u>						



PROJEC		CLIENT		INDING LO				
FROJEC	& New Force Main				Jacobs			
	Manatee County, Florida Project No.: DES 218808	WATER	TABLE:	3.5'		DATE:	0/5/21	
TECHNIC	CIAN:	DATE:			COMPLE	TION DEP	TH:	
LOCATIC	<u>N.P./K.A.</u>	TEST N	10/5/ UMBER:	21		10.0'		
	See Plate I-B		T T		HA-2 HAND CO			
ELEV. (FT)	DESCRIPTION	DEPTH (FT)	SYMBOL		RESISTAN	CE (TSF)		_
	Dark grayish-brown Fine SAND with roots (SP) (A-3)	0		<u> </u>	20 30	40 50	0 60	) 7
,	Dark grayish-brown slightly silty Fine SAND with trace of roots (SP-SM) (A-3)							
	Dark reddish-brown slightly organic, slightly silty Fine SAND(SP-SM)(A-3)	- 2	11)11(11) 11)11(11) 11)11(11) 11)11(11)	•			+	
	Light brown Fine SAND (SP) (A-3)							
		- 4						
		- 6					+	
		- 8		×				
	Brown Fine SAND (SP) (A-3)						+	
-		10					• +	
	Note: Borehole Grouted							
		- 12						
	LEGEND:							
	• + Denotes Penetration Resistance in excess of 50 TSF	- 14						



PROJEC	HAND AUGER BORING/	CLIENT							
PROJEC	& New Force Main				Ja	cobs			
	Manatee County, Florida	WATER	TABLE:	3.7'			DA	TE: 10/5	/21
TECHNI	Project No.: DES 218808	DATE:				COMP	LETION	DEPTH:	:
LOCATI	N.P./K.A.	TEST NU	10/	5/21			1	0.0'	
LUCATI	See Plate I-D	TESTING	JWDER.		Н	A-3			
			Ь			HAND C RESISTA			
ELEV. (FT)	DESCRIPTION	DEPTH (FT)	SYMBOL					.,	
( ,				0 10	20	30	40	50	60 7
	Grayish-brown Fine SAND	0	1						
	with trace of roots (SP) (A-3)								
	Brown Fine SAND (SP) (A-3)			•					
	Dark brown slightly silty Fine SAND	<u> </u>							
	(SP-SM) (A-3)		10000						
	Light brown Fine SAND (SP) (A-3)								
				1 1					
	Design all all the sittle Fires CAND	4	11111		┝				
	Brown slightly silty Fine SAND (SP-SM) (A-3)		10.000						
	(31-3)				T				
			1.1.6.6.4		<b>/</b>				
			10-11-11 1701-111	╡ ┥					
		- 6	11111						
		Ŭ	110101.						
					<b>^</b>				
				∮ ⋠	$ \rightarrow $				
	Brown silty Fine SAND (SM) (A-2-4)							+	
		- 8 -					-		
						$\rightarrow$			
		- 10 ·						-++	
	Note: Borehole Grouted								
		- 12 -		┣───┣					
	LEGEND:								
	÷								
	• + Denotes Penetration Resistance		-						
	in excess of 50 TSF	- 14 -							
		14							



PROJECT		CLIENT:				
	& New Force Main Manatee County, Florida	WATER TABL	E:	acobs	DATE:	
TECHNIC	Project No.: DES 218808	DATE:	4.0'	COMPL	10/5/ ETION DEPTH:	21
	N.P./K.A.		0/5/21		10.0'	
LOCATIO	N: See Plate I-D	TEST NOMBE		HA-4		
ELEV. (FT)	DESCRIPTION	DEPTH OBW (FT) S		HAND CO RESISTAN	ICE (TSF)	
1	Dark brown Fine SAND with roots (SP) (A-3) Brown Fine SAND (SP) (A-3)			0 30	40 50	60 7
	Light brown Fine SAND (SP) (A-3)	- 2				
-	Brown slightly silty Fine SAND (SP-SM) (A-3)	- 4 - 1111			+	
	Dark reddish-brown Fine SAND (SP) (A-3)				• +	
		- 6 -			•+	
	Dark reddish-brown slightly silty Fine SAND (SP-SM) (A-3)	20-96-6 17-06-6 12-06-6 12-06-6 12-06-6 12-06-6 12-06-6				
		- 8 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1	i   🦯		•+	
_		10	10 I I		•+	
	Note: Borehole Grouted					
		- 12 -				
	LEGEND:					
	• + Denotes Penetration Resistance					
	in excess of 50 TSF	- 14 -				



	HAND AUGER BORING/	HAND CO	NE SO	UNDING LOC	3		
PROJEC	T: Lift Station 9A Rehabilitation & New Force Main	CLIENT		la	cobs		
	Manatee County, Florida Project No.: <u>DES 218808</u>	WATER	TABLE:		.003	DATE: 10/6	/21
TECHNIC	CIAN: K.A.	DATE:	10/	6/21	COMPLE	TION DEPTH: 10.0'	
LOCATIO	DN:	TEST NU	JMBER:		ـــــــــــــــــــــــــــــــــــــ	10.0	
	See Plate I-F				A-5 HAND CO		
ELEV. (FT)	DESCRIPTION	DEPTH (FT)	SYMBOL	F 0 10 20	RESISTANO	CE (TSF) 40 50	60 70
	Dark brown Fine SAND with roots (SP) (A-3)	0					
l l	Light brown Fine SAND (SP) (A-3)					+	
	Dark grayish-brown Fine SAND with trace of shell fragments (SP) (A-3)	- 2 -				+	
	Light brown Fine SAND (SP) (A-3)					•+	
	Dark grayish-brown Fine SAND (SP) (A-3)					+	
	Tan Fine SAND (SP) (A-3)	4				•+	
						+	
		- 6 -					
						+	
-	Dark brown Fine SAND (SP) (A-3)	- 8 -					
						+	
-		10 -				+ + +	_
	Note: Borehole Grouted						
		- 12 -					
	LEGEND:						
	<ul> <li>+ Denotes Penetration Resistance in excess of 50 TSF</li> </ul>						
		- 14 -					



PROJEC		CLIENT:							
	& New Force Main Manatee County, Florida	WATER	TABLE:		Ja	cobs	DATE:		
TECHNIC	Project No.: DES 218808	DATE:		3.5'		COMP	LETION DEP	<u>0/6/21</u> TH:	
	K.A.	TEST NU	10/6	6/21			10.0'	25 12 200	
LOCATIC	DN: See Plate I-G	TESTING			Η	A-6			
ELEV. (FT)	DESCRIPTION	DEPTH (FT)	SYMBOL			RESISTA	ONE TIP NCE (TSF)		
	Dark brown Fine SAND with roots (SP) (A-3)	0		0 10	0 20	30	40 50	) 60 +	) 70
	Dark brown slightly silty Fine SAND with trace of rock fragments (SP-SM) (A-3)							+++	
	Grayish-brown Fine SAND with trace of shell (SP) (A-3)	- 2 -						+	
	Dark brown slightly silty Fine SAND with roots (SP-SM) (A-3)		ע זין און זין אין						
		- 4 -							
	Dark brown slightly silty Fine SAND (SP-SM) (A-3)							+	
	Brown slightly silty Fine SAND	6 -						+	
	(SP-SM) (A-3)		73.010 446644 117.010	•					
		- 8 -							
	Tan Fine SAND (SP) (A-3)							++	
-		- 10 -				$\leq$		+	
	Note: Borehole Grouted								
		- 12 -							
	LEGEND:								
	<ul> <li>+ Denotes Penetration Resistance in excess of 50 TSF</li> </ul>	- 14 -							



	HAND AUGER BORING	IAND CONE SOUNDING LOG	
PROJE	CT: Lift Station 9A Rehabilitation	CLIENT:	
	& New Force Main Manatee County, Florida Project No.: DES 218808	Jacobs WATER TABLE: DATE: 3.4' DATE:	1
TECHNI	CIAN: K.A.	DATE: COMPLETION DEPTH: 10/6/21 10.0'	
LOCATI	ON: See Plate I-H	TEST NUMBER: HA-7	
ELEV. (FT)	DESCRIPTION	HAND CONE TIP DEPTH (FT)	60 70
	Dark grayish-brown Fine SAND with roots (SP) (A-3) Grayish-brown slightly silty Fine SAND with trace of rock fragments (SP-SM) (A-3) Dark grayish-brown Fine SAND (SP) (A-3)		60 70
	Light brown Fine SAND (SP) (A-3)		
	Brown slightly silty Fine SAND (SP-SM) (A-3)		
	Dark brown slightly silty Fine SAND (SP-SM) (A-3)	8 111 1 1 111 1 1 1	
	Note: Borehole Grouted		
		- 12 -	
	LEGEND:		
	<ul> <li>+ Denotes Penetration Resistance in excess of 50 TSF</li> </ul>	- 14 -	



PROJE	HAND AUGER BORING/	CLIENT		UNDING	LUG				
FROSE	& New Force Main				Jacobs	5			
	Manatee County, Florida Project No.: DES 218808	WATER	TABLE:	3.4'			DATE: 1	0/6/21	
TECHNI	CIAN:	DATE:	10/	6/21	C	OMPLET	TION DEP 10.0'	TH:	
LOCATI		TEST N	UMBER:	0/21			10.0		
	See Plate I-I				<u>HA-8</u> HA				
ELEV. (FT)	DESCRIPTION	DEPTH (FT)	SYMBOL		RES	ISTANC	E (TSF)		70
	Dark grayish-brown Fine SAND with roots (SP) (A-3)	0		0 10	20	30	40 50	60	70
	Brown Fine SAND (SP) (A-3)								
	Dark grayish-brown Fine SAND (SP) (A-3)	2							
	Brown Fine SAND (SP) (A-3)	- 4						+	
	Light brown slightly silty Fine SAND (SP-SM) (A-3)		1100001 110000 110000 110000 110000 110000 1100000					+	
		- 6	1966) 1966) 1997) 1996) 1997) 1997)					+	
	Light grayish-brown Fine SAND	- 8	01 (11 (11 (11 (11 (11 (11 (11 (11 (11 (		$\geq$	•			
	(SP) (A-3)	10						+	
	Note: Borehole Grouted		-						
		- 12	-						
	LEGEND:								
	<ul> <li>+ Denotes Penetration Resistance in excess of 50 TSF</li> </ul>		-						
		- 14	_						_



PROJEC	CT: Lift Station 9A Rehabilitation	CLIENT:	
TROJEC	& New Force Main	Jacobs	
	Manatee County, Florida Project No.: DES 218808	WATER TABLE: DATE: 3.7'	10/6/21
TECHNIC	CIAN:	DATE: COMPLETION DE 10/6/21 10.0	PTH:
LOCATIO	K.A	TEST NUMBER:	
	See Plate I-K	HA-9 HAND CONE TIP	
ELEV. (FT)	DESCRIPTION	DEPTH C RESISTANCE (TSF)	50 60 70
	Tan Fine SAND with roots (SP) (A-3)		
	Tan Fine SAND (SP) (A-3) Brown Fine SAND (SP) (A-3)		•
	Note: Borehole Grouted		+
	<ul> <li>+ Denotes Penetration Resistance in excess of 50 TSF</li> </ul>		

SUMMARY OF LABORATORY TEST RESULTS

SL
ESUI
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TEST
ORY
RAT
<b>NBOI</b>
FLA
<b>NOF</b>
MAF
SUM

RES.	(ohm-cm)															e.
SO 4	(mqq)				+											& New Force Main, Manatee County, Florida DES 218808
	(mdd)														oilitation	Aanatee Co
Hq	ĩ														Lift Station 9A Rehabilitation	ce Main, N 8
ORG.	(%)	3.7		8.6		4.0	4.0							Jacobs	ift Station	& New Force DES 218808
G.S.			*		*			*	*					Ja		Ϋ́Ω.
CON.														<b>CLIENT:</b>	PROJECT:	•• 5
u.c.														CLI	PRO	FILE:
P.P.	(tsf)													_		
3ERG TS	Id													ni oilicter		) Sieve
ATTERBERG LIMITS	LL PL				-							 	Test	ent tri		y es g No. 200
G s													Consolidation Test	Organic Content Total Chloride	Total Sulfate	Lao Kesistivity See Test Curves Percent Passing No. 200 Sieve
P A	(pcf)												Con	Orga	Tota	Lao See
% M													ii ii	0.0	11 1	I II II
		ghtly silty Fine SAND		ine SAND	AND	ty Fine SAND	anic,			E.			Con. G S (+1)	ORG. (%) CI. (ppm)	SO4 (ppm)	* 53. (0001-500) **
DESCRIPTION		Dark brown slightly organic, slightly silty Fine SAND	Light brown Fine SAND	Dark gray highly organic, silty Fine SAND with fibrous matter	Light brown slightly silty Fine SAND	Dark brown slightly organic, silty Fine SAND with fibrous matter	Dark reddish-brown slightly organic, slightly silty Fine SAND	Brown silty Fine SAND	Brown slightly silty Fine SAND				Water Content Drv Density	Specific Gravity Liquid Limit	Plastic Limit Plasticity Indev	Pocket Penetrometer Unconfined Compression
DEPTH (ft)	Ê	1.5-2.4	6.0-7.5	3.4-4.4	8.0-9.5	4.6-6.0	1.5-3.0	4.0-7.3	7.3-8.5				Water Drv D	Specia	Plastic	Pocke
BORING NO.		SPT-1	I-T-I	SPT-3	SPT-3	SPT-7	HA-2	HA-3	HA-7				W % = Y d (ncf) =	Gs = LL =	PL = PI =	. (tsf) C.

## **GRAINSIZE ANALYSES**

Driggers Engineering Services Incorporated









## **METHOD OF TESTING**

Driggers Engineering Services Incorporated

## STANDARD PENETRATION TEST AND SOIL CLASSIFICATION

## **STANDARD PENETRATION TEST (ASTM D-1586)**

In the Standard Penetration Test borings, a rotary drilling rig is used to advance the borehole to the desired test depth. A viscous drilling fluid is circulated through the drill rods and bit to stabilize the borehole and to assist in removal of soil and rock cuttings up and out of the borehole.

Upon reaching the desired test depth, the 2 inch O.D. split-barrel sampler or "split-spoon", as it is sometimes called, is attached to an N-size drill rod and lowered to the bottom of the borehole. A 140 pound hammer, attached to the drill string at the ground surface, is then used to drive the sampler into the formation. The hammer is successively raised and dropped for a distance of 30 inches using a rope and "cathead" assembly. The number of blows is recorded for each 6 inch interval of penetration or until virtual refusal is achieved. In the above manner, the samples are ideally advanced a total of 18 inches. The sum of the blows required to effect the final 12 inches of penetration is called the blowcount, penetration resistance or "N" value of the particular material at the sample depth.

After penetration, the rods and sampler are retracted to the ground surface where the core sample is removed, sealed in a glass jar and transported to the laboratory for verification of field classification and storage.

### SOIL SYMBOLS AND CLASSIFICATION

Soil and rock samples secured in the field sampling operation were visually classified as to texture, color and consistency. The Unified Soil Classification was assigned to each soil stratum per ASTM D-2487. Soil classifications are presented descriptively and symbolically for ease of interpretation. The stratum identification lines represent the approximate boundary between soil types. In many cases, this transition may be gradual.

Consistency of the soil as to relative density or undrained shear strength, unless otherwise noted, is based upon Standard Penetration resistance values of "N" values and industry-accepted standards. "N" values, or blowcounts, are presented in both tabular and graphical form on each respective boring log at each sample interval. The graphical plot of blowcount versus depth is for illustration purposes only and does not warrant continuity in soil consistency or linear variation between sample intervals.

The borings represent subsurface conditions at respective boring locations and sample intervals only. Variations in subsurface conditions may occur between boring locations. Groundwater depths shown represent water depths at the dates and time shown only. The absence of water table information does not necessarily imply that groundwater was not encountered.

Rev. 9/2011

### HAND CONE PENETRATION TEST

The cone penetration test was performed using a DGSI Model S-215 double rod Static Cone Penetrometer.

Dual rods enable the cone stress to be measured directly. Soil friction on the outer rod does not influence the reading. Depending upon the application, either the maximum bearing for an increment of push or the least bearing for an increment can be reported. If you were investigating for soft spots, you would take the least reading. In typical use, you would force the cone into the soil 6 inches, retract the cone slightly until the gauge reads zero, then advance an additional 6 inch increment. If you meet with refusal, the cone can be removed and the hole opened with a hand auger to permit a continuation of measurements against depth.

The tool has been designed to allow a maximum force of 250 lbs. to be applied, somewhat more than the average weight of an operator. The unit can be operated in a vertical or horizontal position. The cone tip has an included angle of 60 degrees. The cone has a section area of  $1.5 \text{ cm}^2$ . The maximum total bearing (Q<sub>c</sub>) is 70 kg/cm<sup>2</sup>.

The reading  $(Q_c)$  is in kg/cm<sup>2</sup> which is essentially equal to ton/ft<sup>2</sup>.

The cone index  $(Q_c)$  is read directly. The correlation between the cone index and soil constants is not absolute. Generally, the following results have been determined through extensive field use of the unit. Further verification of correlation in your local soil types is essential.

Standard Penetration (Sands)	Strength and Cohesion	
N=Standard Penetration Test Blowcount	Q <sub>u</sub> - Unconfined com c - Cohesion (kg/cn	
$Q_{c} = 4N$	Uniform clay and silty clays:	$Q_{c} = 5 Q_{u}$ $Q_{c} = 10 c$
	Clayey Silts:	$Q_{c} = (10 \text{ to } 20) Q_{u}$ $Q_{c} = (20 \text{ to } 40) c$