

## SITE CONDITIONS

Our scope of services was developed based on this understanding of the project, so the details below should be verified. Aspects of the project that are undefined or assumed at this point are highlighted as shown here in the following table. We request input from the design team to verify any such information as noted.

Item	Description		
Project information	The existing water main along the following roadways is to be replaced:  US-41 from 69 <sup>th</sup> Ave. W. to approximately 400 feet south of 69 <sup>th</sup> Ave. W.  69 <sup>th</sup> Ave. W. from US-41 to 11 <sup>th</sup> St. W.  11 <sup>th</sup> St. W. from 69 <sup>th</sup> Ave. W. to 67 <sup>th</sup> Ave. Terr. W.  67 <sup>th</sup> Ave. Terr. W. from 11 <sup>th</sup> St. W. to 8 <sup>th</sup> St. Ct. W.  8 <sup>th</sup> St. Ct. W. from 67 <sup>th</sup> Ave. Terr. W. to 67 <sup>th</sup> Ave. Dr. W.  67 <sup>th</sup> Ave. Dr. W. from 8 <sup>th</sup> St. Ct. W. to 5 <sup>th</sup> St. E.  5 <sup>th</sup> St. E. from 67 <sup>th</sup> Ave. Dr. W. to 63 <sup>rd</sup> Ave. W.  Total approximate length = 6,900 linear feet  See Exhibit 1: Site Location (See Exhibit 1: Site Location)		
Existing improvements	Asphalt paved roadways with grassed shoulders		
Current ground cover	Asphalt pavement, short grasses, and bare earth		
Existing topography  The northeast end of the site is at an elevation of about +18 ½ to + NAVD88 and slopes downward to the southwest end of the site at a elevation of about +10 feet-NAVD88. Site grades are to remain relationships.			



#### **EXPLORATION AND TESTING PROCEDURES**

Based on our understanding of the project as noted in **Project Understanding**, and as requested by you, we completed the following scope of services for field exploration and laboratory testing for this project.

## **Field Exploration**

Our field exploration work included the drilling and sampling of exploratory soil borings consistent with the following schedule.

Number of Borings	Boring Depth (ft)	Planned Location
22	8	Water Main
2	16	Jack and Bore Locations

Locations of soil borings are provided on Exhibit 2A through 2G: Anticipated Exploration Plan. The locations were established in the field by Terracon's exploration team using a measuring wheel/tape and/or a hand-held GPS unit with reference to known points. The two 16-foot deep borings will be located in the vicinity of the planned Jack and Bore. The accuracy of the exploration points is usually within 10 feet of the noted location. The ground surface elevations are estimated from the most recent USGS topographic maps, and the accuracy of the ground surface at each point is probably about 2 feet.

We advanced the soil borings with a truck-mounted drill rig using a cutting head and stabilizing with the use of bentonite (drillers' mud). We obtained representative samples primarily by the split-barrel sampling procedure. In the split-barrel sampling procedure, a standard, 2-inch O.D., split-barrel sampling spoon is driven into the boring with a 140-pound rope and cathead operate SPT (Standard Penetration Test) hammer falling 30 inches. We recorded the number of blows required to advance the sampling spoon the middle 12 inches of a 24-inch sampling interval as the standard penetration resistance value, N.

Our exploration team prepared field boring logs as part of the drilling operations. These field logs include visual classifications of the materials encountered during drilling and driller's interpretation of the subsurface conditions between samples. Ground water observations were also recorded. The final boring logs included with this report represent the engineer's interpretation of the field logs and include modifications based on observations and tests of the samples in the laboratory.



## **Laboratory Testing**

The project engineer reviewed the field data and assigned various laboratory tests to better understand the engineering properties of the various soil and rock strata as necessary for this project. Procedural standards noted below are for reference to methodology in general. In some cases, variations to methods are applied as a result of local practice or professional judgment. Standards noted below include reference to other, related standards. Such references are not necessarily applicable to describe the specific test performed.

- ASTM D2216-10: Standard Test Methods for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass
- ASTM D422-63(2007)e2: Standard Test Method for Particle-Size Analysis of Soils
- ASTM D2974-04: Standard Test Method for Organic Content
- Standard Test Method for laboratory determination of pH (EPA 9045C), resistivity (ASTM D1125), sulfate content (EPA 9056), and chloride content (EPA 300.0)

The laboratory testing program also included examination of soil samples by an engineer. Based on observation and test data, the engineer classified the soil samples in accordance with the Unified Soil Classification System (ASTM D2487). Additionally, nine (9) samples were transported to Palm Beach Environmental Laboratories, Inc. for corrosion series testing (pH, resistivity, sulfate content, and chloride content).



#### **GEOTECHNICAL MODEL**

Subsurface conditions on the project site can be generalized as follows:

Stratum	Approximate Depth to Bottom of Stratum	Material Description	Consistency/Density
4	4 to 6 inches	Asphalt pavement	Not applicable
ı	7 to 11 inches	Sand-shell base course	Not applicable
21	4 feet	Organic SAND, with silt	Medium dense
3	4 to 16 feet	Fine SAND with trace to slight amounts of silt, trace shell fragments, and occasionally trace to some organic material (SP, SP-SM)	Very loose to dense
42	16 feet	Weathered LIMESTONE	Very hard

- 1. Only found in Boring B-10 at a depth of 2 to 4 feet bgs.
- 2. Only found in Boring B-15 at a depth of 15  $\frac{1}{2}$  to 16 feet bgs.

Conditions encountered at each boring location are indicated on the individual boring logs. Stratification boundaries on the boring logs represent the approximate location of changes in soil types; in situ, the transition between materials may be gradual. Details for each of the borings can be found in Exploration Results. A discussion of field sampling and laboratory testing procedures and test results are presented in Exploration and Testing Procedures.

The percent by weight of the organic content identified in samples from Strata 2 and 3 soils are presented below. Generally, soils with an organic content greater than 5% are not suitable for pipe backfill or bedding.

Boring No.	Depth of the layer	Organic Content (%)
B-4	4 to 6 feet	4.8
B-7	6 to 8 feet	1.2
B-10	2 to 4 feet	7.8
B-16	4 to 6 feet	3.7
B-18	0 to 2 feet	4.8



#### Groundwater

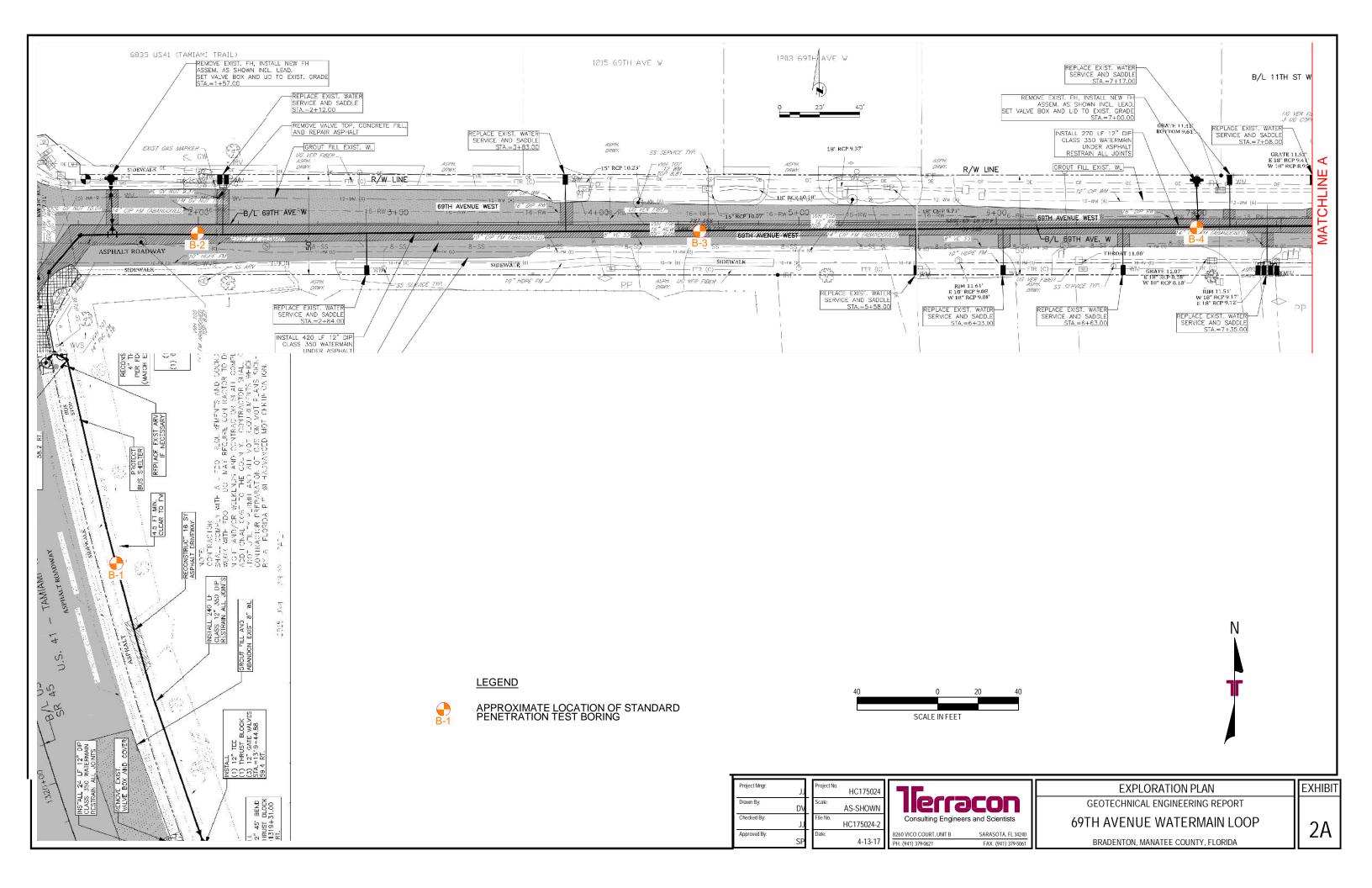
The boreholes were observed while drilling for the presence and level of groundwater. The water levels observed in the boreholes can be found in **Exploration Results**, and are summarized below.

Boring number	Depth to groundwater while drilling, ft.	Boring number	Depth to groundwater while drilling, ft.
B-1	8	B-13	5 ½
B-2	8	B-14	8
B-3	8	B-15	5
B-4	8	B-16	4 ½
B-5	4 ½	B-17	4 ½
B-6	5	B-18	7 ½
B-7	5	B-19	4 ½
B-8	6	B-20	4 ½
B-9	5	B-21	5
B-10	3 ½	B-22	5 ½
B-11	3 ½	B-23	4 ½
B-12	3 ½	B-24	5

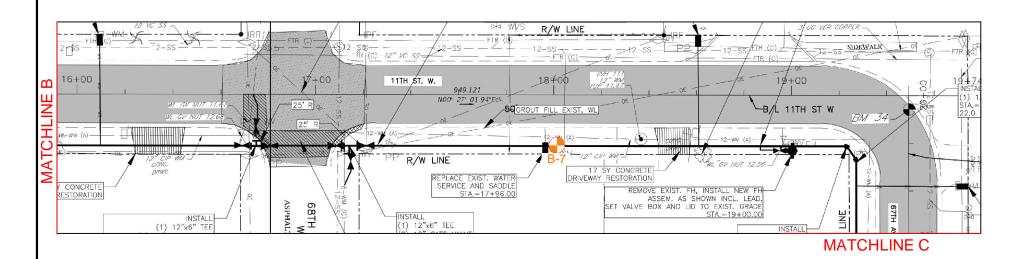
The groundwater measurements are influenced by the drilling process and ambient weather conditions which have been very dry.

Groundwater level fluctuations occur due to seasonal variations in the amount of rainfall, runoff and other factors not evident at the time the borings were performed. Therefore, groundwater levels during construction or at other times in the life of the structure may be higher or lower than the levels indicated on the boring logs. The possibility of groundwater level fluctuations should be considered when developing the design and construction plans for the project.

If a more detailed seasonal high groundwater level estimate is needed, we recommend the installation of shallow groundwater monitoring wells (i.e. piezometers) for the collection of stabilized groundwater level measurements.



#### **MATCHLINE A** R/W LINE ORWY. 15-SS TR (Q) 15-SS 15 - SS - 15 - S 12" VC 55 FTR (C)-11TH ST. W. 12+00 13+00 14+00 15+00 B/L 11TH ST W. 949.121 N00° 27' 01.94'E VH 170 12" WM 70P 11.53 -BM 33 -25' R NSTALL 260 LF DR-18 PVC 16 SY CONCRETE DRIVEWAY RESTORATION W 18" RCP 8,96' E 18" RCP 8,96' 3 UC VER COPPER -REMOVE VALVE BOX, CONCRETE FILL, AND REPAIR ASPHALT 25 DRIVEWAY INSTALL (1) 12"x8" TEE (1) 12" GATE VALVE (1) 8" GATE VALVE 85 SY BASE, MILL AND RESURFACE 251 SY MILL AND RESURFACE 25 LF CURB RESTORATION INSTALL 500 LF 12" DR-18 PVC WATERMAIN REPLACE EXIST, WATER— SERVICE AND SADDLE STA.=12+60.04



**LEGEND** 

APPROXIMATE LOCATION OF STANDARD PENETRATION TEST BORING





Project Mngr:	JJ	Project No. HC175024	
Drawn By:	DV	Scale: AS-SHOWN	
Checked By:	JJ	File No. HC175024-2	
Approved By:	SP	Date: 4-13-17	82 PI

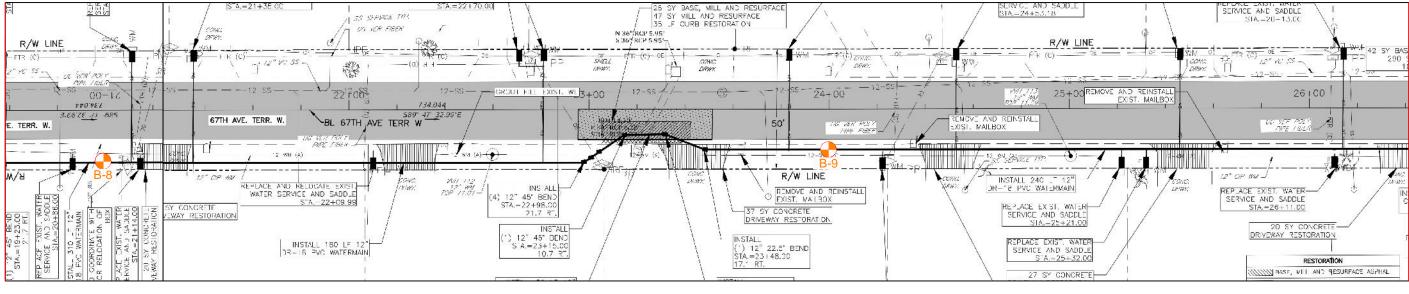


EXPLORATION PLAN
GEOTECHNICAL ENGINEERING REPORT
69TH AVENUE WATERMAIN LOOP
BRADENTON, MANATEE COUNTY, FLORIDA

2B

EXHIBIT





**LEGEND** 

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MATCHLINE

APPROXIMATE LOCATION OF STANDARD PENETRATION TEST BORING

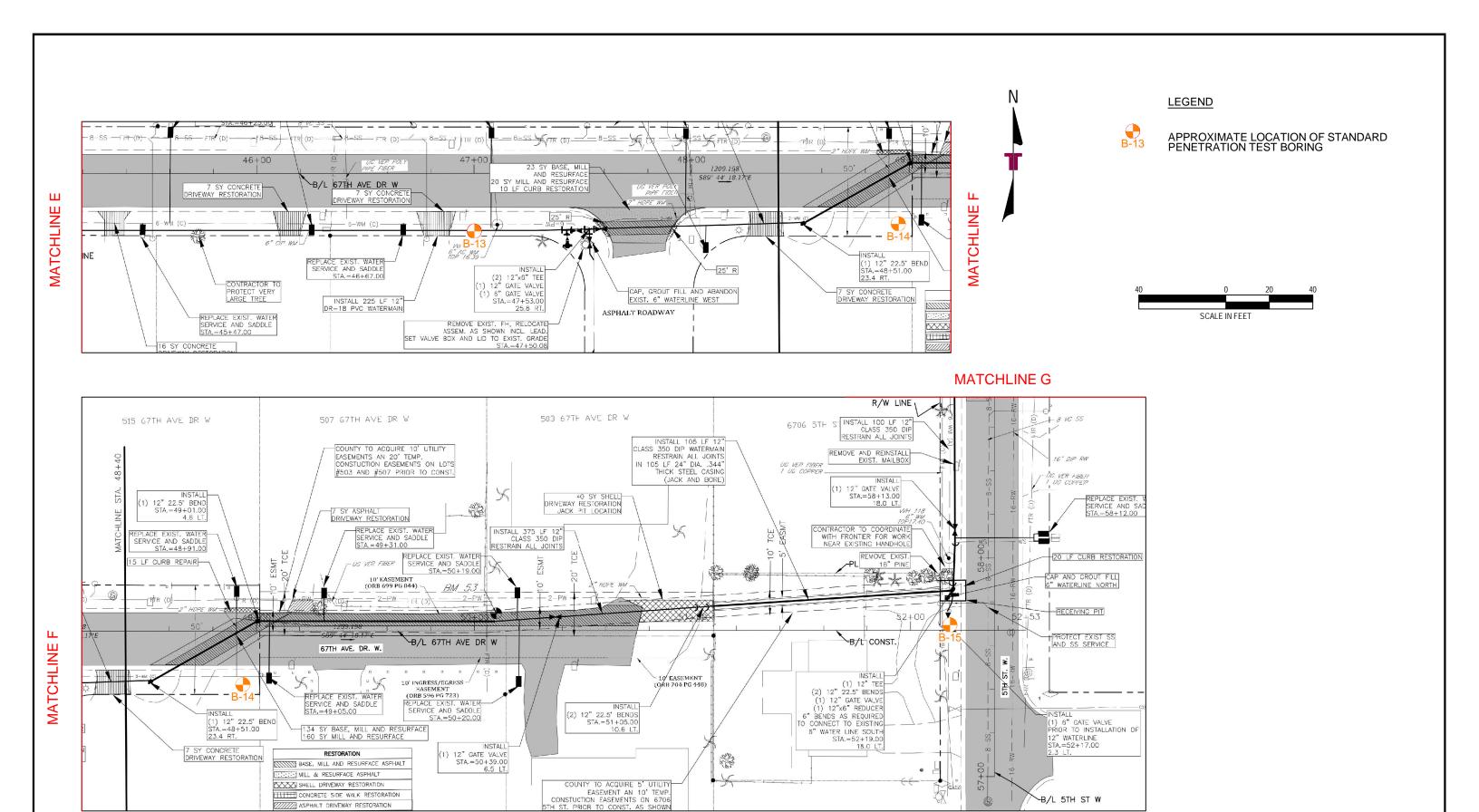




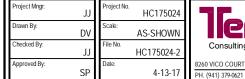
Project Mngr:	IJ	Project No.	HC175024	
Drawn By:	DV	Scale:	AS-SHOWN	
Checked By:	JJ	File No.	HC175024-2	Consult
Approved By:	SD	Date:	4-13-17	8260 VICO COU
	3P		4-13-1/	PH. (941) 379-06

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N	lietta	CON	
2	Consulting Engineer	s and Scientists	
_	8260 VICO COURT, UNIT B	SARASOTA, FL 34240	l

EXHIBIT **EXPLORATION PLAN** GEOTECHNICAL ENGINEERING REPORT 69TH AVENUE WATERMAIN LOOP BRADENTON, MANATEE COUNTY, FLORIDA



ASPHALT DRIVEWAY RESTORATION



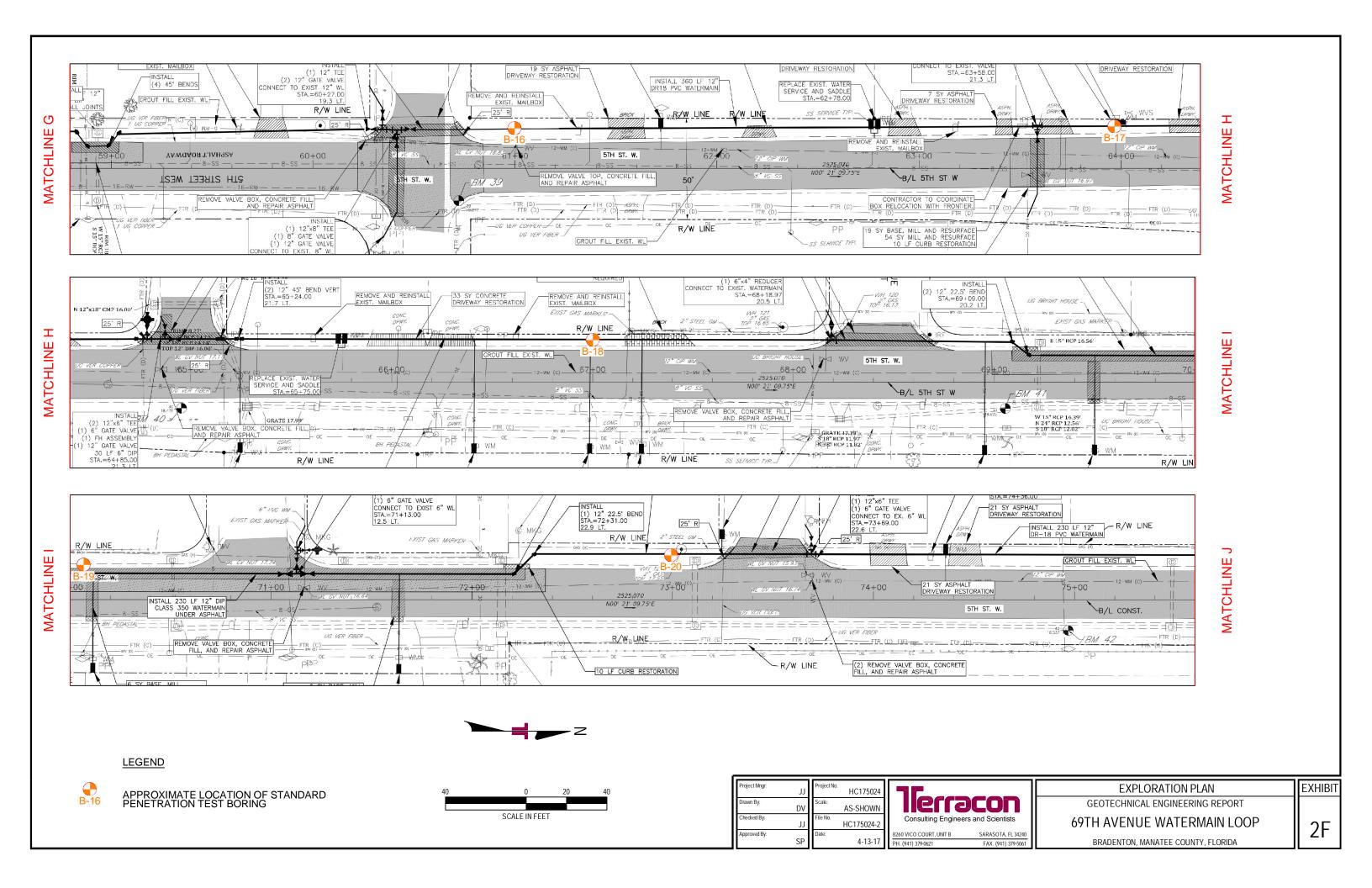


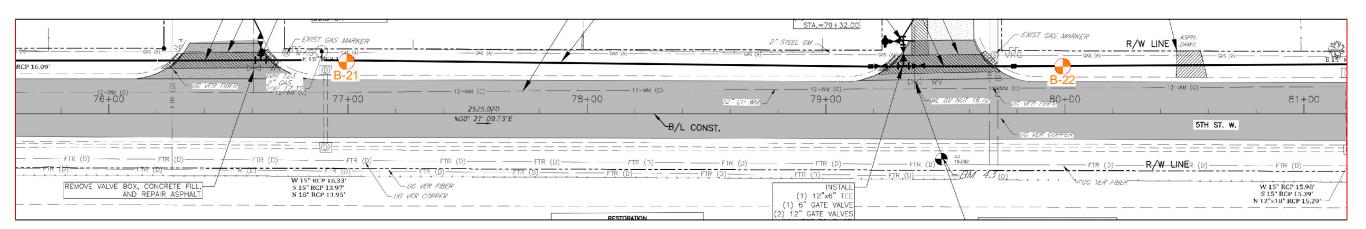
FAX. (941) 379-506

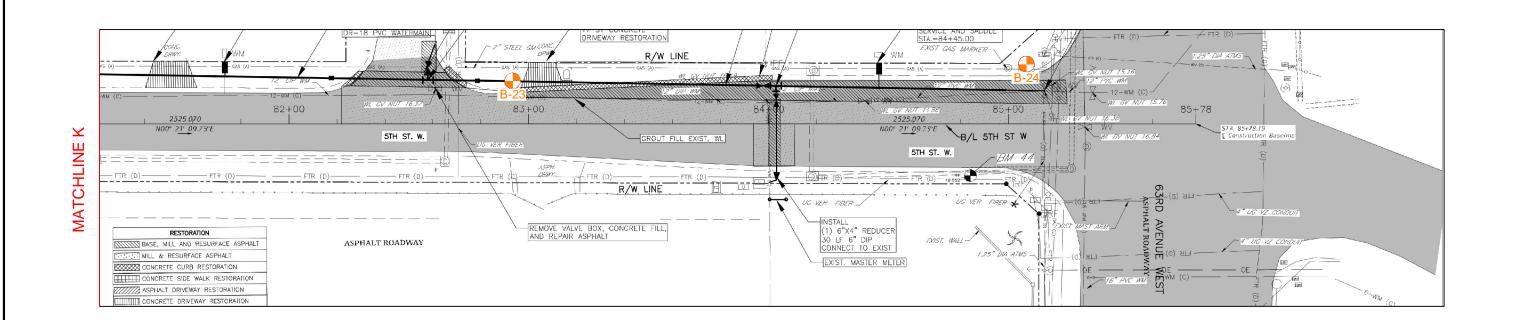
**EXPLORATION PLAN** GEOTECHNICAL ENGINEERING REPORT 69TH AVENUE WATERMAIN LOOP BRADENTON, MANATEE COUNTY, FLORIDA

2E

EXHIBIT







**LEGEND** 

B-21

 $\neg$ 

MATCHLINE

APPROXIMATE LOCATION OF STANDARD PENETRATION TEST BORING





Project Mngr:	IJ	Project No.	HC175024	
Drawn By:	DV	Scale:	AS-SHOWN	
Checked By:	JJ	File No.	HC175024-2	Cor
Approved By:	SP	Date:	4-13-17	8260 VICC

1	Terra Consulting Engineers	CON s and Scientists	
1	8260 VICO COURT, UNIT B	SARASOTA, FL 34240	ı

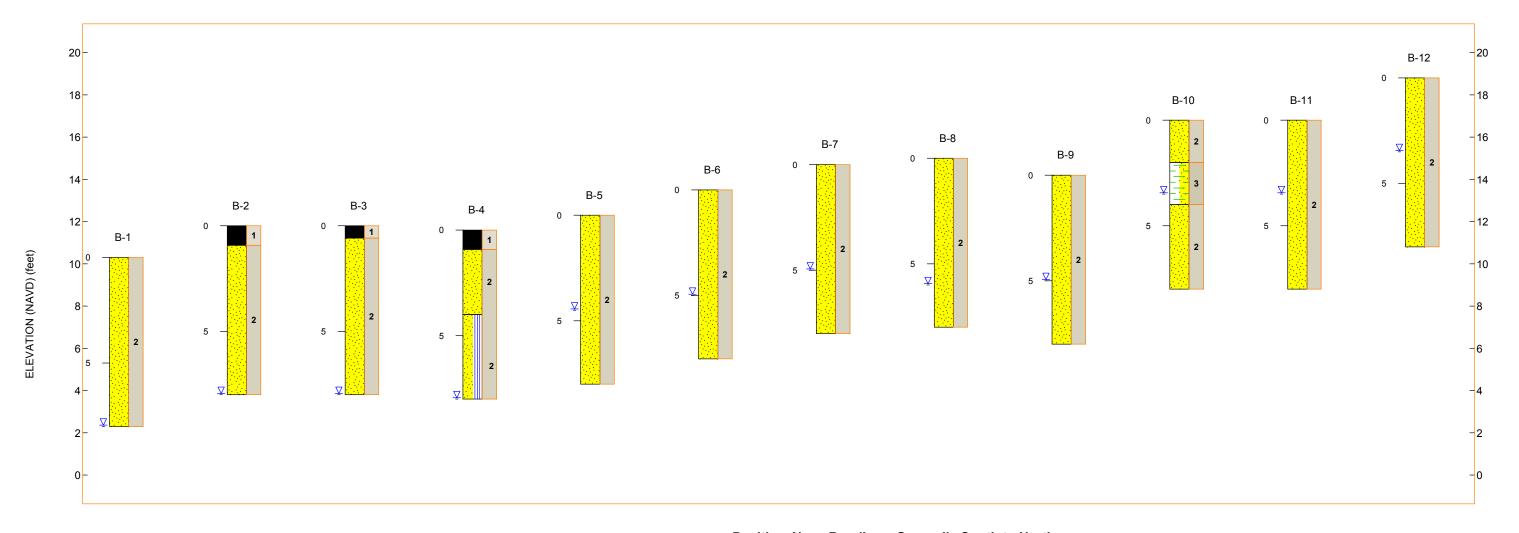
EXPLORATION PLAN
GEOTECHNICAL ENGINEERING REPORT
69TH AVENUE WATERMAIN LOOP
BRADENTON, MANATEE COUNTY, FLORIDA

EXHIBIT

## **GEOTECHNICAL MODEL**







## Position Along Baseline - Generally South to North



Model Layer	Termed	General Description
1	PAVEMENT	4 to 6 inches of Asphalt 6 to 11 inches of Aggregate Base
2	SP, SP-SM	Fine SAND with trace to slight amounts of silt, trace shell fragments, and occasionally trace to some organic material
3	ORGANIC SAND	Organic SAND, with silt
4	Limestone	Weathered Limestone

NOTES:

See boring logs for more detailed conditions specific to each boring.

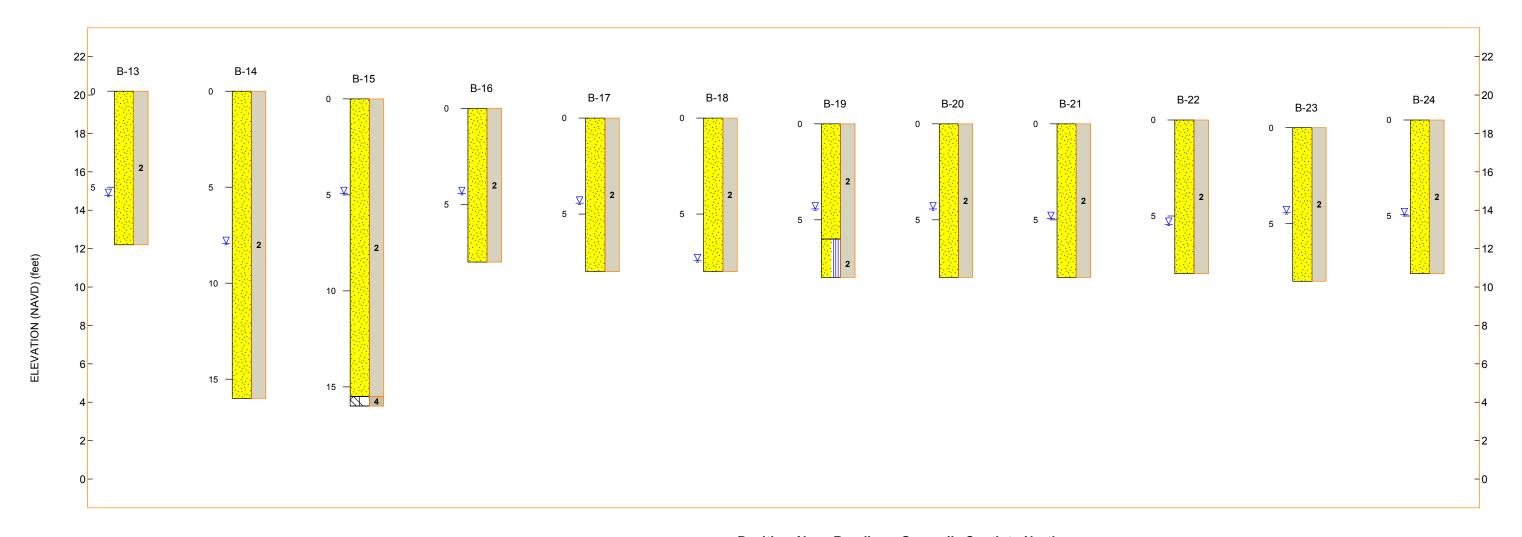
GeoModel provided for illustration purposes only. Actual subsurface conditions between borings will vary.

Layering shown on this figure has been developed by the geotechnical engineer for purposes of characterization of subsurface conditions as required for the subsequent geotechnical engineering for this project.

□ Grounwater observation during drilling

# GEOTECHNICAL MODEL 69th Avenue Watermain Loop ■ Bradenton, FL 5/18/2017 ■ Terracon Project No. HC175024





## Position Along Baseline - Generally South to North





Model Layer	Termed	General Description
1	PAVEMENT	4 to 6 inches of Asphalt 6 to 11 inches of Aggregate Base
2	SP, SP-SM	Fine SAND with trace to slight amounts of silt, trace shell fragments, and occasionally trace to some organic material
3	ORGANIC SAND	Organic SAND, with silt
4	Limestone	Weathered Limestone

NOTES:

See boring logs for more detailed conditions specific to each boring. GeoModel provided for illustration purposes only. Actual subsurface conditions between borings will vary.

Layering shown on this figure has been developed by the geotechnical engineer for purposes of characterization of subsurface conditions as required for the subsequent geotechnical engineering for this project.

LEGEND

## **GENERAL NOTES**

#### **DESCRIPTION OF SYMBOLS AND ABBREVIATIONS**

		$\square$		Water Initially Encountered		(HP)	Hand Penetrometer
	Auger	Split Spoon		Water Level After a Specified Period of Time		(T)	Torvane
NG	Challey Tube	Maara Cara	LEVEL	Water Level After a Specified Period of Time	ESTS	(b/f)	Standard Penetration Test (blows per foot)
IPLIN	Shelby Tube	Macro Core	<u>~</u>	Water levels indicated on the soil boring logs are the levels measured in the	D TE	(PID)	Photo-Ionization Detector
SAMPI	Ring Sampler	Rock Core	WATE	borehole at the times indicated. Groundwater level variations will occur over time. In low permeability soils,	FIEL	(OVA)	Organic Vapor Analyzer
	S. S.			accurate determination of groundwater levels is not possible with short term water level observations.			
	Grab Sample	No Recovery					

#### **DESCRIPTIVE SOIL CLASSIFICATION**

Soil classification is based on the Unified Soil Classification System. Coarse Grained Soils have more than 50% of their dry weight retained on a #200 sieve; their principal descriptors are: boulders, cobbles, gravel or sand. Fine Grained Soils have less than 50% of their dry weight retained on a #200 sieve; they are principally described as clays if they are plastic, and silts if they are slightly plastic or non-plastic. Major constituents may be added as modifiers and minor constituents may be added according to the relative proportions based on grain size. In addition to gradation, coarse-grained soils are defined on the basis of their in-place relative density and fine-grained soils on the basis of their consistency.

#### **LOCATION AND ELEVATION NOTES**

Unless otherwise noted, Latitude and Longitude are approximately determined using a hand-held GPS device. The accuracy of such devices is variable. Surface elevation data annotated with +/- indicates that no actual topographical survey was conducted to confirm the surface elevation. Instead, the surface elevation was approximately determined from topographic maps of the area.

	RELATIVE DENSITY OF COARSE-GRAINED SOILS (More than 50% retained on No. 200 sieve.) Density determined by Standard Penetration Resistance			CONSISTENCY OF FINE-GRAINED SOILS  (50% or more passing the No. 200 sieve.)  Consistency determined by laboratory shear strength testing, field visual-manual procedures or standard penetration resistance				
RMS	Descriptive Term (Density)	Standard Penetration or N-Value Blows/Ft.	Ring Sampler Blows/Ft.	Descriptive Term (Consistency)	Unconfined Compressive Strength, Qu, psf	Standard Penetration or N-Value Blows/Ft.	Ring Sampler Blows/Ft.	
뿌	Very Loose	0 - 3	0 - 6	Very Soft	less than 500	0 - 1	< 3	
NGTH	Loose	4 - 9	7 - 18	Soft	500 to 1,000	2 - 4	3 - 4	
TREN	Medium Dense	10 - 29	19 - 58	Medium-Stiff	1,000 to 2,000	4 - 8	5 - 9	
ြလ	Dense	30 - 50	59 - 98	Stiff	2,000 to 4,000	8 - 15	10 - 18	
	Very Dense	> 50	<u>&gt;</u> 99	Very Stiff	4,000 to 8,000	15 - 30	19 - 42	
				Hard	> 8,000	> 30	> 42	

#### RELATIVE PROPORTIONS OF SAND AND GRAVEL

#### Descriptive Term(s) **Major Component** Percent of Particle Size of other constituents of Sample **Dry Weight** Trace < 15 Boulders Over 12 in. (300 mm) 15 - 29 With Cobbles 12 in. to 3 in. (300mm to 75mm) Modifier > 30 Gravel 3 in. to #4 sieve (75mm to 4.75 mm) Sand #4 to #200 sieve (4.75mm to 0.075mm Silt or Clay Passing #200 sieve (0.075mm)

**GRAIN SIZE TERMINOLOGY** 

PLASTICITY DESCRIPTION

#### **RELATIVE PROPORTIONS OF FINES**

Descriptive Term(s) of other constituents	Percent of Dry Weight	<u>Term</u>	Plasticity Index	
of other constituents	<u>Dry weight</u>	Non-plastic	0	
Trace	< 5	Low	1 - 10	
With	5 - 12	Medium	11 - 30	
Modifier	> 12	High	> 30	



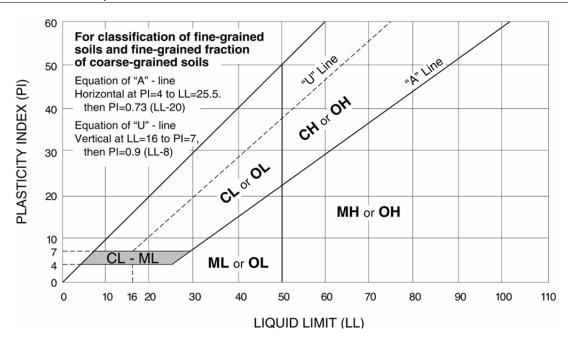
## UNIFIED SOIL CLASSIFICATION SYSTEM

					Soil Classification	
Criteria for Assign	ning Group Symbols	and Group Names	s Using Laboratory Tests A	Group Symbol	Group Name <sup>B</sup>	
	Gravels:	Clean Gravels:	Cu ≥ 4 and 1 ≤ Cc ≤ 3 <sup>E</sup>	GW	Well-graded gravel F	
	More than 50% of	Less than 5% fines <sup>c</sup>	Cu < 4 and/or 1 > Cc > 3 <sup>E</sup>	GP	Poorly graded gravel F	
	coarse fraction retained	Gravels with Fines:	Fines classify as ML or MH	GM	Silty gravel F,G,H	
Coarse Grained Soils: More than 50% retained	on No. 4 sieve	More than 12% fines <sup>C</sup>	Fines classify as CL or CH	GC	Clayey gravel F,G,H	
on No. 200 sieve	Sands: 50% or more of coarse fraction passes No. 4 sieve	Clean Sands:	Cu ≥ 6 and 1 ≤ Cc ≤ 3 <sup>E</sup>	SW	Well-graded sand I	
011110. 200 01010		Less than 5% fines D	Cu < 6 and/or 1 > Cc > 3 <sup>E</sup>	SP	Poorly graded sand I	
		Sands with Fines: More than 12% fines <sup>D</sup>	Fines classify as ML or MH	SM	Silty sand G,H,I	
			Fines classify as CL or CH	SC	Clayey sand G,H,I	
	Silts and Clays: Liquid limit less than 50	Inorganic:	PI > 7 and plots on or above "A" line J	CL	Lean clay K,L,M	
			PI < 4 or plots below "A" line J	ML	Silt K,L,M	
		Organic:	Liquid limit - oven dried	OL	Organic clay K,L,M,N	
Fine-Grained Soils:			Liquid limit - not dried		Organic silt K,L,M,O	
50% or more passes the No. 200 sieve	Silts and Clays: Liquid limit 50 or more	Inorganic:	PI plots on or above "A" line	CH	Fat clay K,L,M	
			PI plots below "A" line	MH	Elastic Silt K,L,M	
		Organia	Liquid limit - oven dried < 0.75	ОН	Organic clay K,L,M,P	
		Organic:	Liquid limit - not dried < 0.75		Organic silt K,L,M,Q	
Highly organic soils:	Primarily organic matter, dark in color, and organic odor			PT	Peat	

<sup>&</sup>lt;sup>A</sup> Based on the material passing the 3-inch (75-mm) sieve

<sup>E</sup> 
$$Cu = D_{60}/D_{10}$$
  $Cc = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ 

Q PI plots below "A" line.





<sup>&</sup>lt;sup>B</sup> If field sample contained cobbles or boulders, or both, add "with cobbles or boulders, or both" to group name.

Gravels with 5 to 12% fines require dual symbols: GW-GM well-graded gravel with silt, GW-GC well-graded gravel with clay, GP-GM poorly graded gravel with silt, GP-GC poorly graded gravel with clay.
 Sands with 5 to 12% fines require dual symbols: SW-SM well-graded

<sup>&</sup>lt;sup>D</sup> Sands with 5 to 12% fines require dual symbols: SW-SM well-graded sand with silt, SW-SC well-graded sand with clay, SP-SM poorly graded sand with silt, SP-SC poorly graded sand with clay

 $<sup>^{\</sup>text{F}}$  If soil contains  $\geq$  15% sand, add "with sand" to group name.

<sup>&</sup>lt;sup>G</sup> If fines classify as CL-ML, use dual symbol GC-GM, or SC-SM.

<sup>&</sup>lt;sup>H</sup> If fines are organic, add "with organic fines" to group name.

<sup>&</sup>lt;sup>1</sup> If soil contains ≥ 15% gravel, add "with gravel" to group name.

J If Atterberg limits plot in shaded area, soil is a CL-ML, silty clay.

K If soil contains 15 to 29% plus No. 200, add "with sand" or "with gravel," whichever is predominant.

 $<sup>^{\</sup>text{L}}$  If soil contains  $\geq$  30% plus No. 200 predominantly sand, add "sandy" to group name.

If soil contains  $\geq$  30% plus No. 200, predominantly gravel, add "gravelly" to group name.

<sup>&</sup>lt;sup>N</sup> PI ≥ 4 and plots on or above "A" line.

 $<sup>^{\</sup>circ}$  PI < 4 or plots below "A" line.

P PI plots on or above "A" line.

## **DESCRIPTION OF ROCK PROPERTIES**

	WEATHERING			
Term	Description			
Unweathered	No visible sign of rock material weathering, perhaps slight discoloration on major discontinuity surfaces.			
Slightly weathered	Discoloration indicates weathering of rock material and discontinuity surfaces. All the rock material may be discolored by weathering and may be somewhat weaker externally than in its fresh condition.			
Moderately weathered	Less than half of the rock material is decomposed and/or disintegrated to a soil. Fresh or discolored rock is present either as a continuous framework or as corestones.			
Highly weathered	More than half of the rock material is decomposed and/or disintegrated to a soil. Fresh or discolored rock is present either as a discontinuous framework or as corestones.			
Completely weathered	All rock material is decomposed and/or disintegrated to soil. The original mass structure is still largely intact.			
Residual soil	All rock material is converted to soil. The mass structure and material fabric are destroyed. There is a large change in volume, but the soil has not been significantly transported.			

STRENGTH OR HARDNESS				
Description	Field Identification	Uniaxial Compressive Strength, PSI (MPa)		
Extremely weak	Indented by thumbnail	40-150 (0.3-1)		
Very weak	Crumbles under firm blows with point of geological hammer, can be peeled by a pocket knife	150-700 (1-5)		
Weak rock	Can be peeled by a pocket knife with difficulty, shallow indentations made by firm blow with point of geological hammer	700-4,000 (5-30)		
Medium strong	Cannot be scraped or peeled with a pocket knife, specimen can be fractured with single firm blow of geological hammer	4,000-7,000 (30-50)		
Strong rock	Specimen requires more than one blow of geological hammer to fracture it	7,000-15,000 (50-100)		
Very strong	Specimen requires many blows of geological hammer to fracture it	15,000-36,000 (100-250)		
Extremely strong	Specimen can only be chipped with geological hammer	>36,000 (>250)		

DISCONTINUITY DESCRIPTION				
Fracture Spacing (J	oints, Faults, Other Fractures)	Bedding Spacing (May Include Foliation or Banding)		
Description	Spacing	Description	Spacing	
Extremely close	< ¾ in (<19 mm)	Laminated	< ½ in (<12 mm)	
Very close	3/4 in - 2-1/2 in (19 - 60 mm)	Very thin	½ in – 2 in (12 – 50 mm)	
Close	2-1/2 in - 8 in (60 - 200 mm)	Thin	2 in – 1 ft (50 – 300 mm)	
Moderate	8 in – 2 ft (200 – 600 mm)	Medium	1 ft – 3 ft (300 – 900 mm)	
Wide	2 ft - 6 ft (600 mm - 2.0 m)	Thick	3 ft – 10 ft (900 mm – 3 m)	
Very Wide	6 ft – 20 ft (2.0 – 6 m)	Massive	> 10 ft (3 m)	

<u>Discontinuity Orientation (Angle)</u>: Measure the angle of discontinuity relative to a plane perpendicular to the longitudinal axis of the core. (For most cases, the core axis is vertical; therefore, the plane perpendicular to the core axis is horizontal.) For example, a horizontal bedding plane would have a 0 degree angle.

ROCK QUALITY DESIGNATION (RQD*)				
Description	RQD Value (%)			
Very Poor	0 - 25			
Poor	25 – 50			
Fair	50 – 75			
Good	75 – 90			
Excellent	90 - 100			

<sup>\*</sup>The combined length of all sound and intact core segments equal to or greater than 4 inches in length, expressed as a percentage of the total core run length.

Reference: U.S. Department of Transportation, Federal Highway Administration, Publication No FHWA-NHI-10-034, December 2009

<u>Technical Manual for Design and Construction of Road Tunnels – Civil Elements</u>

